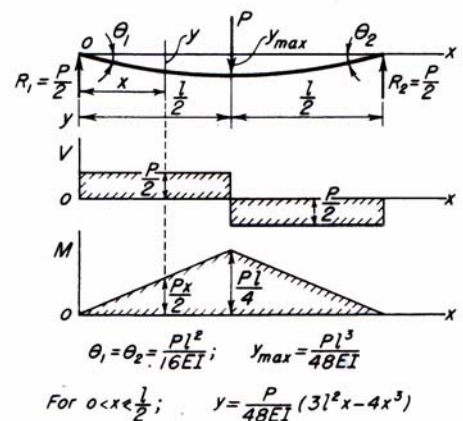
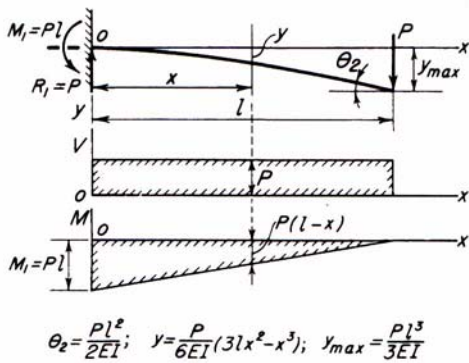
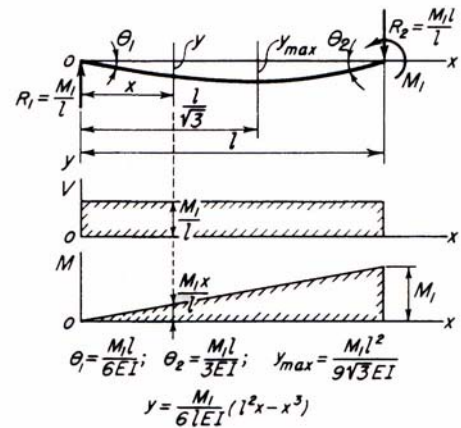
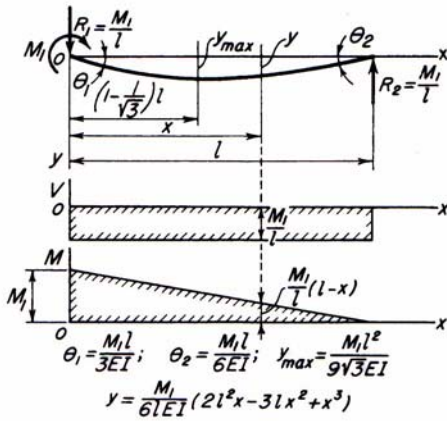
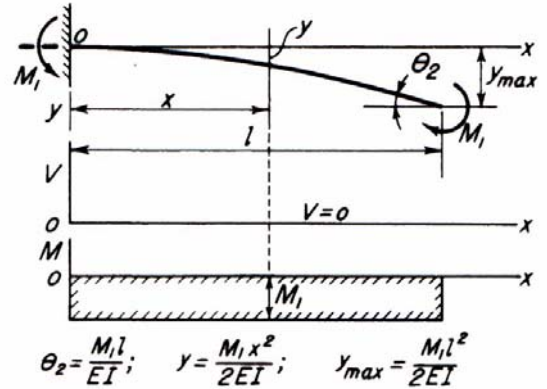
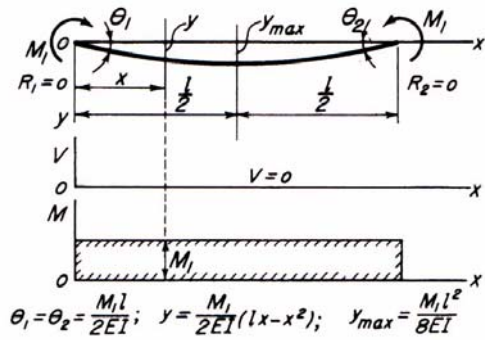
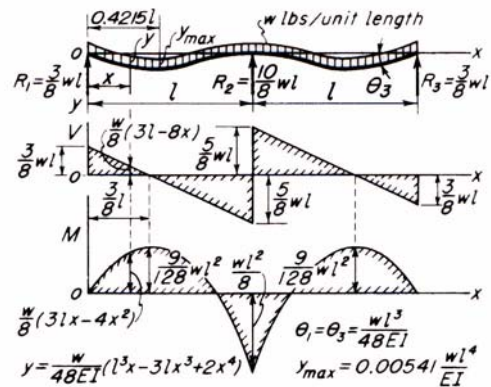
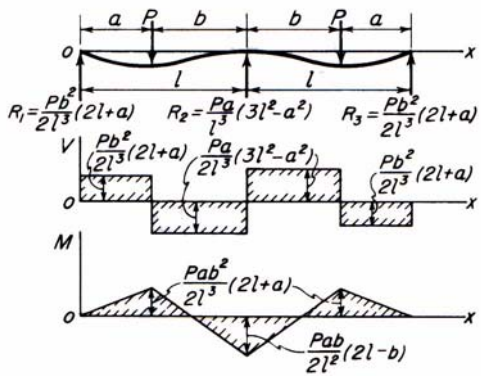
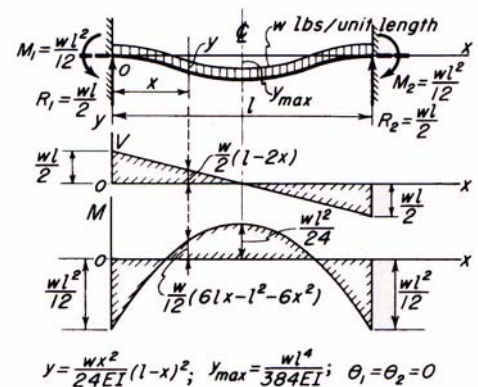
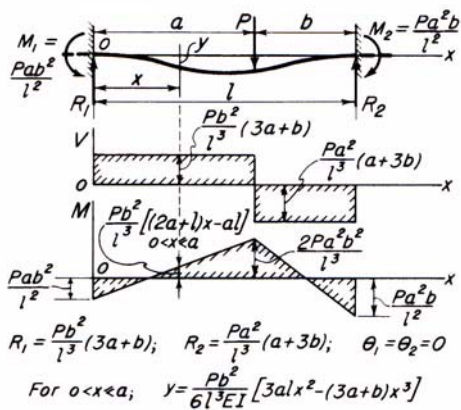
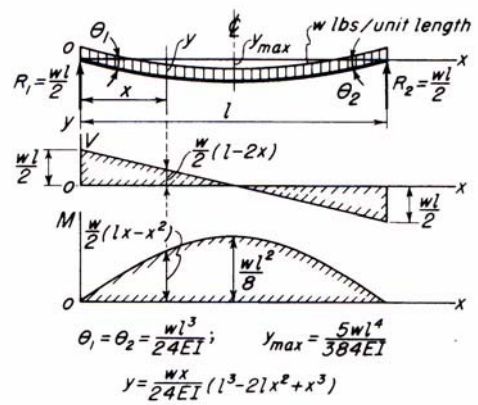
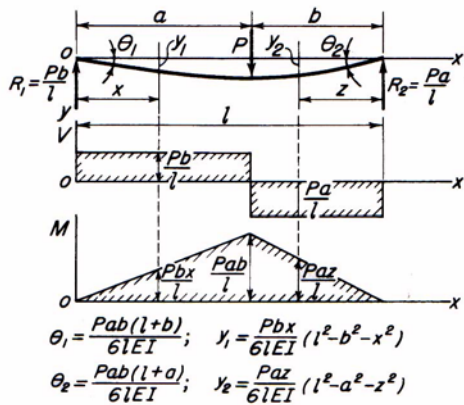
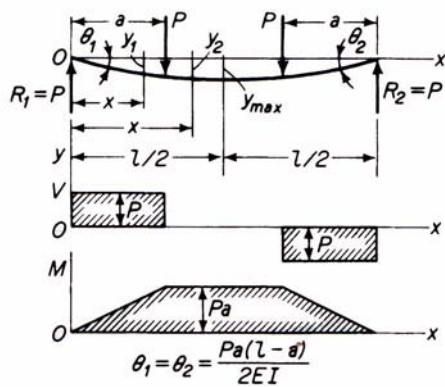


Διαγράμματα διατμητικών δυνάμεων και καμπτικών ροπών σε δοκούς διαφόρων φορτίσεων

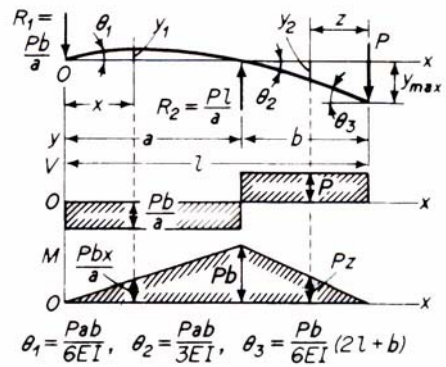






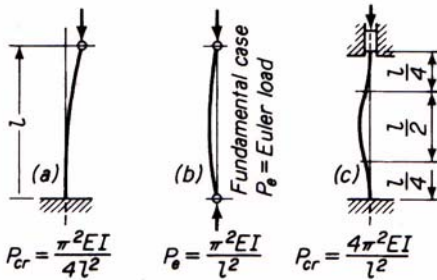
$$\theta_1 = \theta_2 = \frac{Pa(L-a)}{2EI}$$

For $0 < x \leq a$, $y_1 = \frac{Px}{6EI} [3a(l-a) - x^2]$
 For $a \leq x \leq (l-a)$, $y_2 = \frac{Pa}{6EI} [3x(l-x) - a^2]$
 $y_{max} = \frac{Pa}{24EI} (3l^2 - 4a^2)$

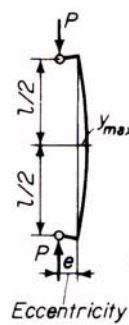


$$\theta_1 = \frac{Pab}{6EI}, \theta_2 = \frac{Pab}{3EI}, \theta_3 = \frac{Pb}{6EI}(2l+b)$$

For $0 < x \leq a$; $y_1 = \frac{Pbx}{6aEI} (x^2 - a^2)$
 For $0 < z \leq b$; $y_2 = \frac{P}{6EI} [z^3 - b(2l+b)z + 2b^2l]$
 $y_{max} = \frac{Pb^2l}{3EI}$



Critical or buckling loads for centrally loaded columns.

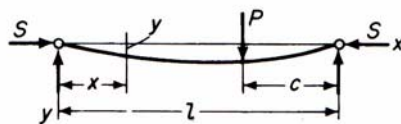


$$y_{max} = e \left(\sec \frac{l}{2} \sqrt{\frac{P}{EI}} - 1 \right)$$

Maximum moment:
 $M_{max} = P(e + y_{max})$
 $= Pe \sec \frac{l}{2} \sqrt{\frac{P}{EI}}$

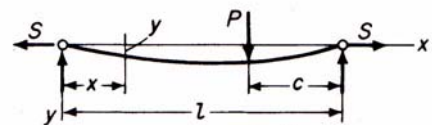
Maximum stress:
 $s_{max} = \frac{P}{A} \left(1 + \frac{ec}{i^2} \sec \frac{l}{2} \sqrt{\frac{P}{AE}} \right)$

Where A = area of cross section
 $i = \sqrt{I/A}$, radius of gyration
 c = distance from neutral axis to edge of section



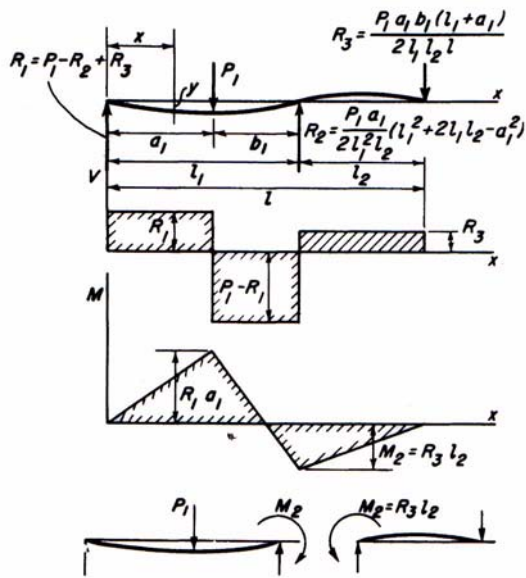
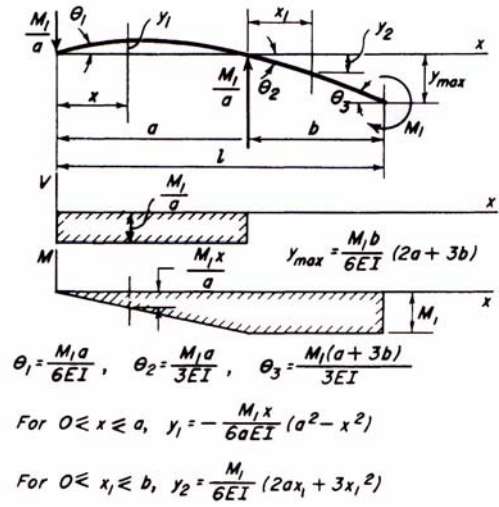
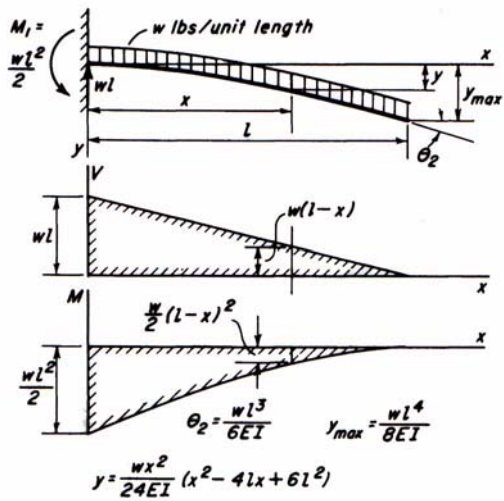
For $0 < x \leq (l-c)$ $p^2 = \frac{S}{EI}$
 $y = \frac{P \sin pc}{Sp \sin pl} \sin px - \frac{Pc}{Sl} x$

Equations for slope and moment can be found by differentiation.

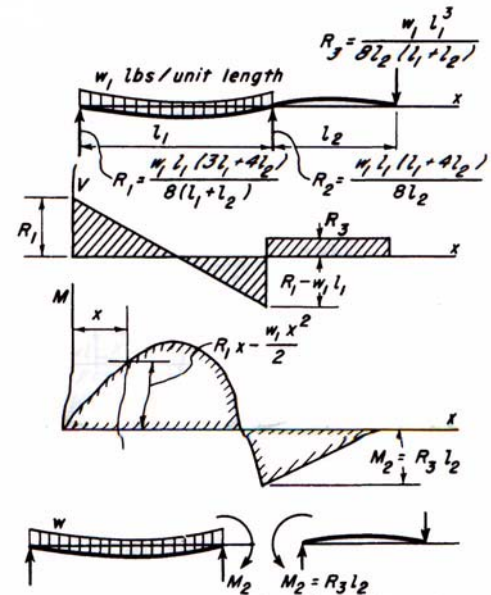


For $0 < x \leq (l-c)$ $p^2 = \frac{S}{EI}$
 $y = -\frac{P \sinh pc}{Sp \sinh pl} \sinh px + \frac{Pc}{Sl} x$

Equations for slope and moment can be found by differentiation.



Deflections can be found by elementary equations and superpositions



Deflections can be found by elementary equations and superposition