

FREE VIBRATION RESPONSE: MDOF SYSTEMS

SYSTEMS WITHOUT DAMPINGEquation of Motion:

$$\mathbf{m}\ddot{\mathbf{u}} + \mathbf{k}\mathbf{u} = \mathbf{0}$$

A very important case in the study of vibrations of MDOF systems is that in which all the coordinates, *i.e.*, all components $u_i(t)$ ($i = 1, 2, \dots, N$) of $\mathbf{u}(t)$, execute the same motion in time.

In other words, the system vibrates maintaining the overall shape of these coordinates/displacements/deflections and changing only their amplitude by a proportionality factor.

In this case, the system is said to execute *synchronous motion*.

To examine the possibility that such motions exist, we consider the solution of the Equation of Motion (see above) in the exponential form:

$$\mathbf{u}(t) = e^{st} \boldsymbol{\phi}$$

where: $\begin{cases} s & \text{is a constant scalar} \\ \boldsymbol{\phi} & \text{is a constant } N\text{-vector} \end{cases}$

Introducing the solution $\mathbf{u}(t) = e^{st} \boldsymbol{\phi}$ in the Equation of Motion we obtain:

$$\left. \begin{array}{l} \mathbf{m}\ddot{\mathbf{u}} + \mathbf{k}\mathbf{u} = \mathbf{0} \\ \mathbf{u}(t) = e^{st} \boldsymbol{\phi} \end{array} \right\} \Rightarrow e^{st} (s^2 \mathbf{m}\boldsymbol{\phi} + \mathbf{k}\boldsymbol{\phi}) = \mathbf{0} \Rightarrow \mathbf{k}\boldsymbol{\phi} = -s^2 \mathbf{m}\boldsymbol{\phi}$$

Therefore:

$$\mathbf{k}\boldsymbol{\phi} = \lambda \mathbf{m}\boldsymbol{\phi} \quad , \quad \lambda = -s^2$$

The above matrix equation represents a set of N simultaneous homogeneous algebraic equations in the unknowns $\boldsymbol{\phi} \stackrel{\text{def}}{=} \{\phi_i\}$ ($i = 1, 2, \dots, N$), with λ playing the role of a parameter.

The problem of determining the values of the parameter λ for which the matrix equation $\mathbf{k}\boldsymbol{\phi} = \lambda \mathbf{m}\boldsymbol{\phi}$ admits nontrivial solutions $\boldsymbol{\phi}$ is known as the *algebraic* (or *matrix*) *eigenvalue* (or *characteristic-value*) *problem*. [NOTE: The obvious, but trivial, solution is $\boldsymbol{\phi} = \mathbf{0}$, however we are not interested in such a solution because it is not associated with motion.] Another name by which the above problem may be encountered is *generalized characteristic-value problem*.

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

Before we proceed, we summarize the properties of the matrix eigenvalue problem that are demonstrated mathematically:

PROPERTIES OF THE ALGEBRAIC (or MATRIX) EIGENVALUE PROBLEM

- (1) The eigenvalues of an algebraic eigenvalue problem, in which \mathbf{k} & \mathbf{m} are **both symmetric**, and at **least one positive definite**, are **all real**.

[NOTE: For civil engineering structures, both matrices \mathbf{k} & \mathbf{m} are **real, symmetric** and **positive definite**. Indeed, $\mathbf{u}^T \mathbf{k} \mathbf{u} > 0$ for $\mathbf{u} \neq \mathbf{0}$, because civil engineering structures are **restrained** and **stable**, and $\mathbf{u}^T \mathbf{m} \mathbf{u} > 0$ for $\mathbf{u} \neq \mathbf{0}$, because all DOFs associated with zero mass/inertia have been eliminated by **static condensation**.]

- (2) When matrices \mathbf{k} & \mathbf{m} are **both real** and **positive definite**, the eigenvalues are **all positive**, and the corresponding eigenvectors are **all real**.

- (3) When \mathbf{k} is **singular**, **at least one** of the eigenvalues must be **zero**.
When \mathbf{m} is **singular**, **at least one** of the eigenvalues must be **infinite**.

- (4) The eigenvectors are orthogonal with respect to both \mathbf{k} & \mathbf{m} .

For **symmetric** \mathbf{k} & \mathbf{m} matrices:

$$\begin{array}{l} \phi_i^T \mathbf{m} \phi_j = 0 \quad i \neq j \\ \phi_i^T \mathbf{k} \phi_j = 0 \quad i \neq j \end{array}$$

- (5) The eigenvectors of an algebraic eigenvalue problem in which the matrices involved are **symmetric**, including those corresponding to repeated eigenvalues, are all **linearly independent**.

- (6) **Any arbitrary vector** of order N can be expressed as a **superposition** of the eigenvectors of an $N \times N$ symmetrical algebraic eigenvalue problem:

$$\mathbf{u} = \sum_{r=1}^N \phi_r q_r = \Phi \mathbf{q}$$

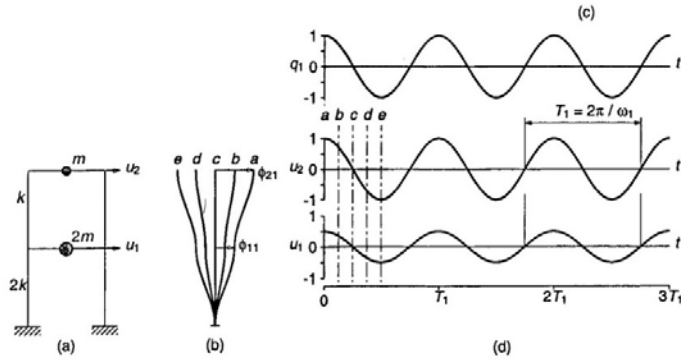
where: $\Phi = \begin{pmatrix} \downarrow & \downarrow & \cdots & \downarrow \\ \phi_1 & \phi_2 & \cdots & \phi_N \\ \downarrow & \downarrow & \cdots & \downarrow \end{pmatrix}$
Modal Matrix

where q_r ($r = 1, 2, \dots, N$) are scalar multipliers called **modal coordinates** or **normal coordinates** and $\mathbf{q} = [q_1 \ q_2 \ \cdots \ q_N]^T$. This theorem is referred to as **modal expansion theorem** or **eigenvector expansion theorem**.

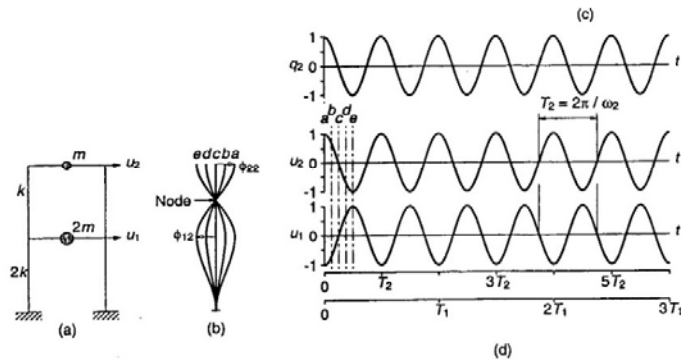
PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

Examples of models of structures and their modal shapes:

Free vibration of undamped system: $\mathbf{u}(0) = \boldsymbol{\phi}_1$



Free vibration of undamped system: $\mathbf{u}(0) = \boldsymbol{\phi}_2$



Free vibration of undamped system: Arbitrary $\mathbf{u}(0)$

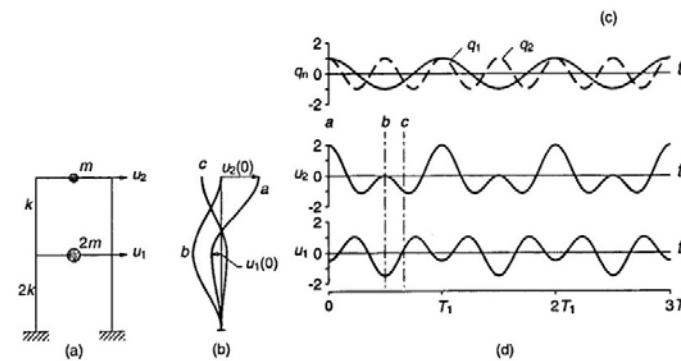


FIGURE: Free vibration of an undamped 2-DOF system.

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

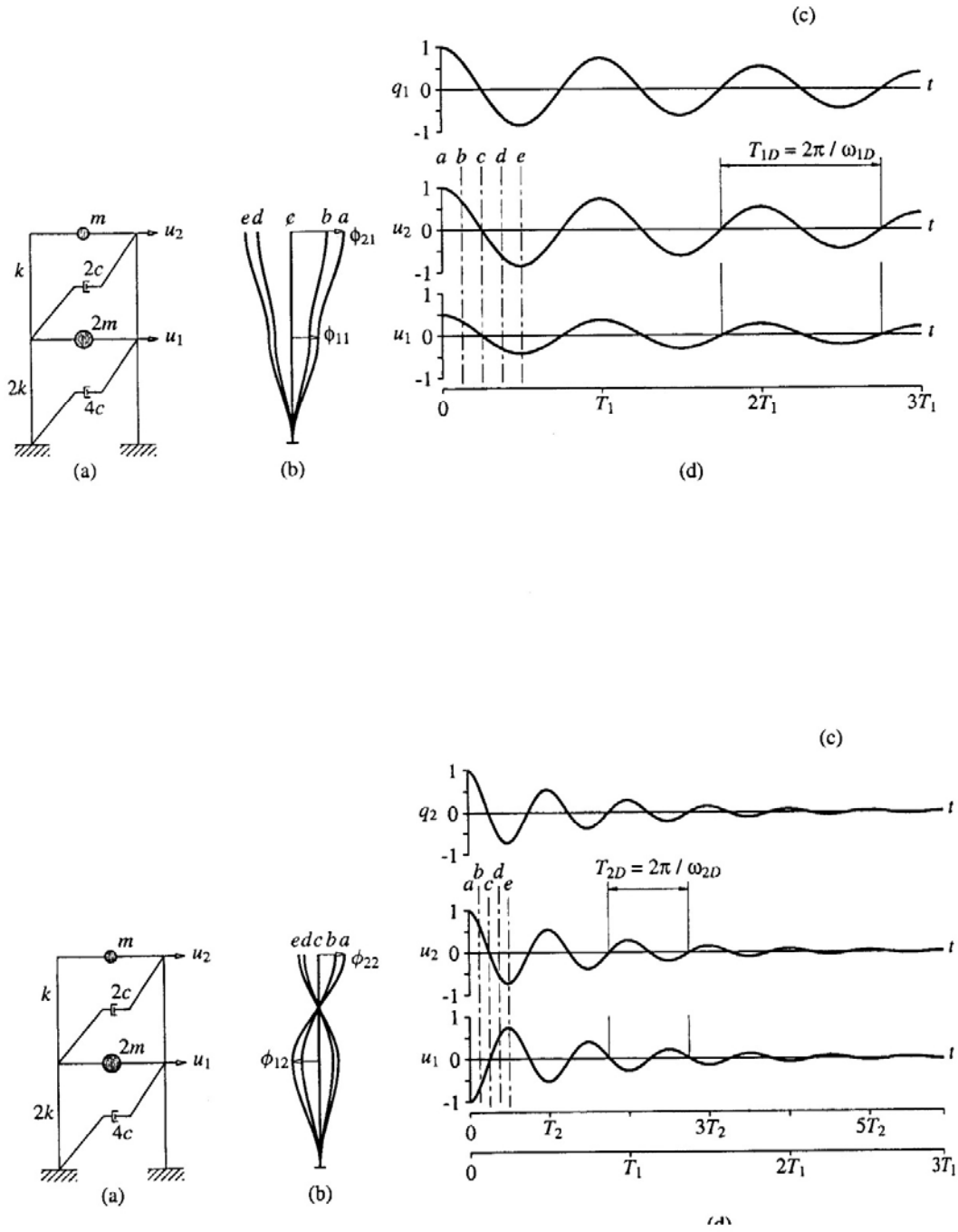


FIGURE: Free vibration of a classically damped 2-DOF system.

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

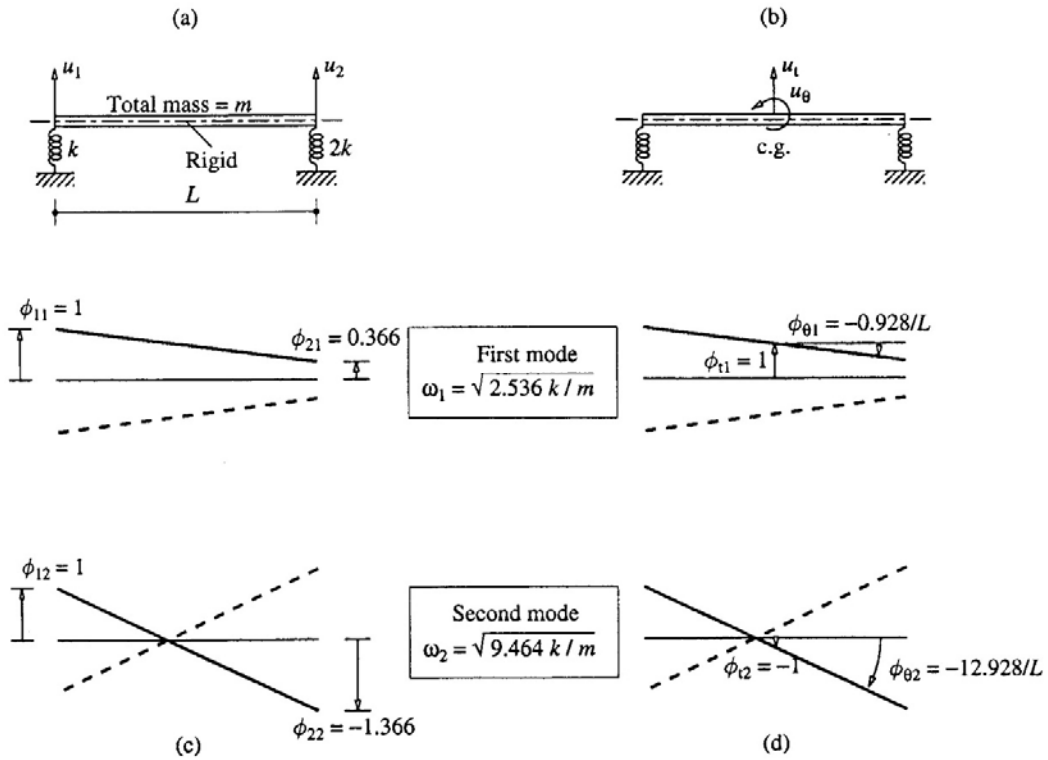


FIGURE: 2-DOF system: Rigid bar on elastic supports.

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

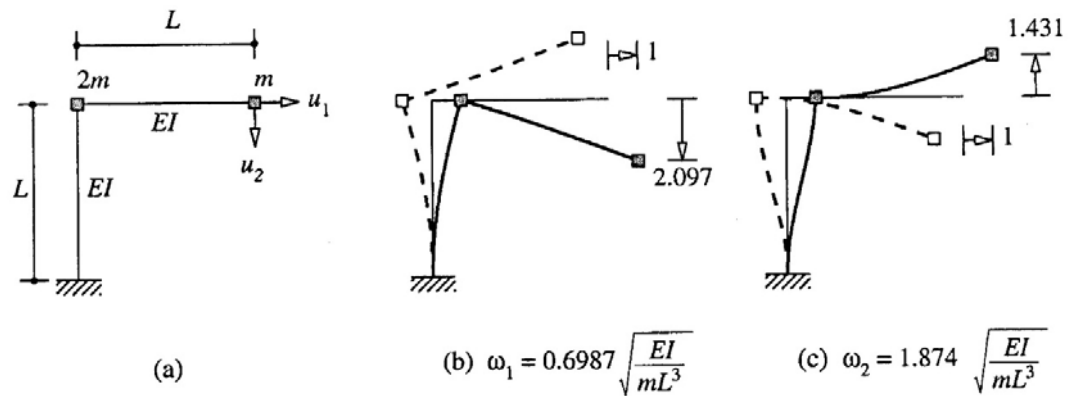
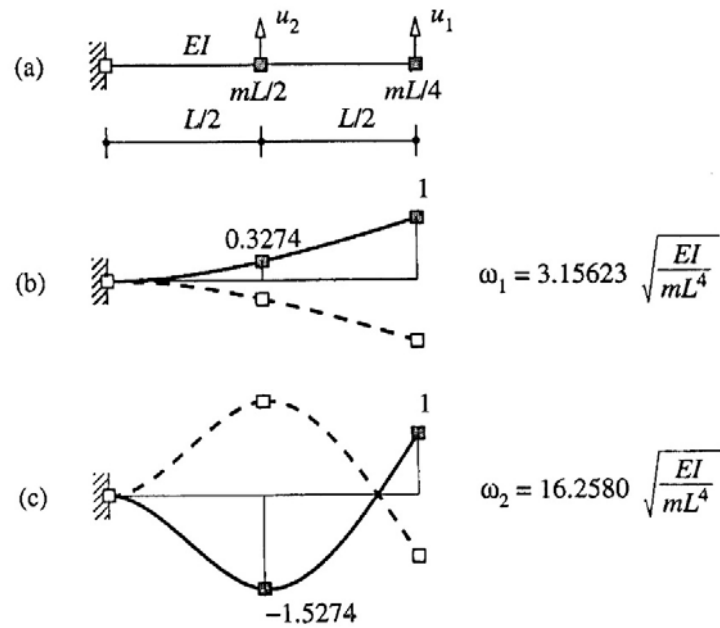


FIGURE: Cantilever beam & L-shaped frame modeled as 2-DOF systems.

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

NOTES:Kinetic and Strain Energy

Consider the discretized model of a structure. Its **strain energy** is given by $V = \left(\frac{1}{2}\right) \mathbf{u}^T \mathbf{k} \mathbf{u}$, while its **kinetic energy** $T = \left(\frac{1}{2}\right) \dot{\mathbf{u}}^T \mathbf{m} \dot{\mathbf{u}}$.

NOTE: The above expressions for the kinetic and strain energies of a discretized structure are justified/obtained as follows:

The forces that are acting on the masses of the structure along the DOFs and accelerate them are equal to: $\frac{d}{dt}(\mathbf{m}\dot{\mathbf{u}})$. The work produced by moving the masses of the structure from state/position 1 to state/position 2 is equal to: $W_{12}^{(T)} = \int_1^2 \left[\frac{d}{dt}(\mathbf{m}\dot{\mathbf{u}})\right]^T d\mathbf{u} = \int_1^2 \left[\frac{d}{dt}(\mathbf{m}\dot{\mathbf{u}})\right]^T \dot{\mathbf{u}} dt = \int_1^2 d\left(\frac{1}{2}\dot{\mathbf{u}}^T \mathbf{m} \dot{\mathbf{u}}\right) = T_2 - T_1$, where T_i ($i = 1, 2$) is the kinetic energy at state/position i .

In an analogous fashion, the strain energy of the structure may be introduced in terms of the work performed by the elastic forces $\mathbf{f}_S = \mathbf{k}\mathbf{u}$ as the structure is deformed from state/position 1 to state/position 2:

$$W_{12}^{(V)} = \int_1^2 \mathbf{f}_S^T d\mathbf{u} = \int_1^2 (\mathbf{k}\mathbf{u})^T d\mathbf{u} = \int_1^2 d\left(\left(\frac{1}{2}\right) \mathbf{u}^T \mathbf{k} \mathbf{u}\right) = V_2 - V_1.$$

Both energy expressions presented above involve a **scalar function** of the form $f = \mathbf{x}^T \mathbf{A} \mathbf{x}$, where the matrix \mathbf{A} (i.e., either \mathbf{k} or \mathbf{m}) is a **real symmetric** matrix. The scalar function $f = \mathbf{x}^T \mathbf{A} \mathbf{x}$ is known as a **quadratic form**. If for **any** real vector $\mathbf{x} \neq \mathbf{0}$ the value of f is **positive**, i.e., $f = \mathbf{x}^T \mathbf{A} \mathbf{x} > 0 \quad \forall \mathbf{x} \neq \mathbf{0} \in \mathbb{R}^N$, then the quadratic form $f = \mathbf{x}^T \mathbf{A} \mathbf{x}$ is called **positive definite** and the associated matrix \mathbf{A} is referred to as a **positive definite matrix**. We have already argued that **for civil engineering structures both matrices \mathbf{m} & \mathbf{k} are positive definite** (because **civil engineering structures are stable and rigidly attached to the ground** and, additionally, **all DOFs associated with zero mass/inertia have been eliminated by static condensation**). Simply stated, for any deformed shape \mathbf{u} , the strain energy of the structure is positive, i.e., $V = \left(\frac{1}{2}\right) \mathbf{u}^T \mathbf{k} \mathbf{u} > 0$. Similarly, for any velocity vector $\dot{\mathbf{u}}$, the kinetic energy of the structure is positive, i.e., $T = \left(\frac{1}{2}\right) \dot{\mathbf{u}}^T \mathbf{m} \dot{\mathbf{u}} > 0$.

Linear Dependence

- We consider a set of **real vectors** $\mathbf{x}_1, \mathbf{x}_2, \dots, \mathbf{x}_S$ in **linear space** \mathcal{L} and a set of **real scalars** $\alpha_1, \alpha_2, \dots, \alpha_S$. Then, the vector \mathbf{x} given by:

$$\mathbf{x} = \alpha_1 \mathbf{x}_1 + \alpha_2 \mathbf{x}_2 + \dots + \alpha_S \mathbf{x}_S$$

is said to be a **linear combination** of $\mathbf{x}_1, \mathbf{x}_2, \dots, \mathbf{x}_S$ with coefficients $\alpha_1, \alpha_2, \dots, \alpha_S$.

- The vectors $\mathbf{x}_1, \mathbf{x}_2, \dots, \mathbf{x}_S$ are said to be **linearly independent** if the relation:

$$\alpha_1 \mathbf{x}_1 + \alpha_2 \mathbf{x}_2 + \dots + \alpha_S \mathbf{x}_S = \mathbf{0} \quad (1)$$

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

can be satisfied only for the trivial case, *i.e.*, only when all the coefficients $\alpha_1, \alpha_2, \dots, \alpha_S$ are identically zero.

- If relation (1) is satisfied and at least one of the coefficients $\alpha_1, \alpha_2, \dots, \alpha_S$ is different from zero, then the vectors $\mathbf{x}_1, \mathbf{x}_2, \dots, \mathbf{x}_S$ are said to be **linearly dependent**, with the implication that **one vector is linear combination of the remaining $(S - 1)$ vectors**.

Thus, considering vectors in the 3D space, two (non-zero) vectors are **linearly dependent** if they are **collinear** (*i.e.*, parallel), and three (non-zero) vectors are **linearly dependent** if they are **coplanar** (*i.e.*, lie in the same plane).

- The subspace \mathcal{S} of \mathcal{L} consisting of the linear combination of the vectors $\mathbf{x}_1, \mathbf{x}_2, \dots, \mathbf{x}_S$ is called a subspace **spanned** by the vectors $\mathbf{x}_1, \mathbf{x}_2, \dots, \mathbf{x}_S$.

If $\mathcal{S} = \mathcal{L}$, then $\mathbf{x}_1, \mathbf{x}_2, \dots, \mathbf{x}_S$ are said to **span \mathcal{L}** .

How is the concept of 'Linear Independence' relevant to our discussion?

The modal shapes $\boldsymbol{\phi}_1, \boldsymbol{\phi}_2, \dots, \boldsymbol{\phi}_N$ of a civil engineering structure are linearly independent and, consequently, any deformed shape \mathbf{u} of the structure may be expressed as a linear combination of the modal shapes (**modal expansion theorem**). Evidently, the modal shapes $\boldsymbol{\phi}_n$ ($n = 1, 2, \dots, N$) play the same role as the three **basis vectors** $\mathbf{i} = [1, 0, 0]^T$, $\mathbf{j} = [0, 1, 0]^T$, and $\mathbf{k} = [0, 0, 1]^T$, in the 3D space. Any vector $\mathbf{v} = v_x \mathbf{i} + v_y \mathbf{j} + v_z \mathbf{k}$ in the 3D space may be expressed as a linear combination of the above three basis vectors, or in matrix form

$$\mathbf{v} = v_x \mathbf{i} + v_y \mathbf{j} + v_z \mathbf{k} = v_x \begin{Bmatrix} 1 \\ 0 \\ 0 \end{Bmatrix} + v_y \begin{Bmatrix} 0 \\ 1 \\ 0 \end{Bmatrix} + v_z \begin{Bmatrix} 0 \\ 0 \\ 1 \end{Bmatrix} = \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} v_x \\ v_y \\ v_z \end{Bmatrix} = \begin{Bmatrix} v_x \\ v_y \\ v_z \end{Bmatrix}$$

The interested reader may consult the following two references:

HILDEBRAND, F.B. (1965), *Methods of Applied Mathematics*, 2nd Edition, PRENTICE-HALL, Inc.

[Chapter 1: Matrices and Linear Equations]

STRANG, G. (1980). *Linear Algebra and Its Applications*, 2nd Edition, ACADEMIC PRESS, Inc.

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

Since \mathbf{k} & \mathbf{m} are **both positive definite**, it follows that all λ_j ($j = 1, 2, \dots, N$) are **positive**. Then, it is convenient to introduce the notation:

$$\lambda_j = \omega_j^2 \quad (j = 1, 2, \dots, N)$$

where ω_j ($j = 1, 2, \dots, N$) are **real positive numbers**.

Recall that $\lambda = -s^2$. Therefore, $s = \pm\sqrt{-\lambda} = \pm\sqrt{-\omega^2} = \pm i\omega$ we conclude that to each eigenvalue λ_j there corresponds the **pair of pure imaginary complex conjugate exponents**:

$$\left. \begin{matrix} s_j \\ \bar{s}_j \end{matrix} \right\} = \pm i\omega_j \quad (j = 1, 2, \dots, N)$$

Introducing the exponents into the expression $\mathbf{u}(t) = e^{st}\boldsymbol{\phi}$, we conclude that the equation of motion $\mathbf{m}\ddot{\mathbf{u}} + \mathbf{k}\mathbf{u} = \mathbf{0}$ admits **synchronous solutions of the form**:

$$\begin{aligned} \mathbf{u}_j(t) &= (C_j e^{i\omega_j t} + \bar{C}_j e^{-i\omega_j t}) \boldsymbol{\phi}_j \\ &= \underbrace{[A_j \cos(\omega_j t) + B_j \sin(\omega_j t)]}_{q_j(t)} \boldsymbol{\phi}_j \\ &= \underbrace{\rho_j \cos(\omega_j t - \theta_j)}_{q_j(t)} \boldsymbol{\phi}_j \quad (j = 1, 2, \dots, N) \\ &= \underbrace{\rho_j \sin(\omega_j t + \vartheta_j)}_{q_j(t)} \boldsymbol{\phi}_j \end{aligned}$$

where: $\rho_j = \text{amplitude}; \rho_j = \sqrt{A_j^2 + B_j^2}$

$\theta_j = \text{phase angle}; \theta_j = \tan^{-1}(B_j/A_j)$ [Consult the notes on SDOF System]

$\vartheta_j = \text{phase angle}; \vartheta_j = \tan^{-1}(A_j/B_j)$ [Consult the notes on SDOF System]

$\omega_j = \text{natural frequency}$ corresponding to **natural mode of vibration** $\boldsymbol{\phi}_j$

($\boldsymbol{\phi}_j$ are also referred to as *eigenvectors*, or *characteristic vectors*, or *natural modes*)

It is straightforward to demonstrate that $\mathbf{u}_j(t) = q_j(t)\boldsymbol{\phi}_j$ satisfies the equation of motion $\mathbf{m}\ddot{\mathbf{u}} + \mathbf{k}\mathbf{u} = \mathbf{0}$. Indeed, by **direct substitution**, $[-\omega_j^2 \mathbf{m}\boldsymbol{\phi}_j + \mathbf{k}\boldsymbol{\phi}_j]q_j(t) = \mathbf{0} \Leftrightarrow \mathbf{k}\boldsymbol{\phi}_j = \omega_j^2 \mathbf{m}\boldsymbol{\phi}_j$.

Clearly, the **synchronous motions** that we have established are **harmonic** (i.e., all DOFs **oscillate harmonically** about their equilibrium position **with the same circular frequency**).

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

FREE VIBRATION RESPONSE – UNDAMPED SYSTEM

We want to solve the **Equation of Motion** $\mathbf{m}\ddot{\mathbf{u}} + \mathbf{k}\mathbf{u} = \mathbf{0}$, subject to the following initial conditions:
 $\mathbf{u}(t = 0) = \mathbf{u}_0$ & $\dot{\mathbf{u}}(t = 0) = \dot{\mathbf{u}}_0$.

We express the solution/response by invoking the **modal expansion theorem**:

$$\begin{aligned}\mathbf{u}(t) &= \sum_{n=1}^N (A_n \cos(\omega_n t) + B_n \sin(\omega_n t)) \boldsymbol{\phi}_n = \sum_{n=1}^N q_n(t) \boldsymbol{\phi}_n \\ \dot{\mathbf{u}}(t) &= \sum_{n=1}^N (-A_n \sin(\omega_n t) + B_n \cos(\omega_n t)) \omega_n \boldsymbol{\phi}_n = \sum_{n=1}^N \dot{q}_n(t) \boldsymbol{\phi}_n\end{aligned}$$

where N is the **total number of DOF** of the model of the discretized structure.

Therefore,

$$\begin{aligned}\mathbf{u}_0 = \mathbf{u}(0) &= \sum_{r=1}^N A_r \boldsymbol{\phi}_r = \sum_{r=1}^N q_r(0) \boldsymbol{\phi}_r \\ \dot{\mathbf{u}}_0 = \dot{\mathbf{u}}(0) &= \sum_{r=1}^N B_r \omega_r \boldsymbol{\phi}_r = \sum_{r=1}^N \dot{q}_r(0) \boldsymbol{\phi}_r\end{aligned}$$

Pre-multiplying, both equations, by $\boldsymbol{\phi}_n^T \mathbf{m}$ and invoking the **eigenvector orthogonality theorem** we obtain:

$$\begin{aligned}\boldsymbol{\phi}_n^T \mathbf{m} \mathbf{u}_0 = A_n \boldsymbol{\phi}_n^T \mathbf{m} \boldsymbol{\phi}_n &\Rightarrow A_n = \frac{\boldsymbol{\phi}_n^T \mathbf{m} \mathbf{u}_0}{\boldsymbol{\phi}_n^T \mathbf{m} \boldsymbol{\phi}_n} = q_n(0) \\ \boldsymbol{\phi}_n^T \mathbf{m} \dot{\mathbf{u}}_0 = B_n \omega_n \boldsymbol{\phi}_n^T \mathbf{m} \boldsymbol{\phi}_n &\Rightarrow B_n \omega_n = \frac{\boldsymbol{\phi}_n^T \mathbf{m} \dot{\mathbf{u}}_0}{\boldsymbol{\phi}_n^T \mathbf{m} \boldsymbol{\phi}_n} = \dot{q}_n(0)\end{aligned}$$

Therefore, the free vibration response of the undamped system is expressed as follows:

$$\boxed{\begin{aligned}\mathbf{u}(t) &= \sum_{n=1}^N q_n(t) \boldsymbol{\phi}_n = \sum_{n=1}^N \left(q_n(0) \cos(\omega_n t) + \frac{\dot{q}_n(0)}{\omega_n} \sin(\omega_n t) \right) \boldsymbol{\phi}_n \\ q_n(0) &= \frac{\boldsymbol{\phi}_n^T \mathbf{m} \mathbf{u}_0}{\boldsymbol{\phi}_n^T \mathbf{m} \boldsymbol{\phi}_n} = \frac{\boldsymbol{\phi}_n^T \mathbf{m} \mathbf{u}_0}{M_n}, \quad \dot{q}_n(0) = \frac{\boldsymbol{\phi}_n^T \mathbf{m} \dot{\mathbf{u}}_0}{\boldsymbol{\phi}_n^T \mathbf{m} \boldsymbol{\phi}_n} = \frac{\boldsymbol{\phi}_n^T \mathbf{m} \dot{\mathbf{u}}_0}{M_n}\end{aligned}}$$

IMPORTANT NOTE: Evidently, for the structure to vibrate only in the n -th mode, the initial displacement \mathbf{u}_0 and/or the initial velocity $\dot{\mathbf{u}}_0$ must both be proportional to $\boldsymbol{\phi}_n$, i.e., $\mathbf{u}_0 \sim \boldsymbol{\phi}_n$ & $\dot{\mathbf{u}}_0 \sim \boldsymbol{\phi}_n$.

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

FREE VIBRATION RESPONSE – SYSTEM WITH DAMPING

Statement of the problem:

$$\begin{array}{l} \text{Equation of Motion:} \quad \mathbf{m}\ddot{\mathbf{u}} + \mathbf{c}\dot{\mathbf{u}} + \mathbf{k}\mathbf{u} = \mathbf{0} \\ \text{Initial Conditions:} \quad \mathbf{u}_0 = \mathbf{u}(0) \quad , \quad \dot{\mathbf{u}}_0 = \dot{\mathbf{u}}(0) \end{array}$$

We are seeking a solution to the above problem. Procedures to obtain the desired solution differ depending on the form of damping: *classical* or *non-classical*; these terms are defined next.

For **civil engineering structures** it is reasonable to assume that the matrices \mathbf{m} , \mathbf{k} , & \mathbf{c} are all *symmetric* and *positive definite*.

It can be demonstrated that the *necessary* and *sufficient* condition for the above system to possess natural modes of vibration, that are **real-valued and identical to those of the associated undamped system** (also referred to as *classical normal modes*), is the following equality:

$$\mathbf{c}\mathbf{m}^{-1}\mathbf{k} = \mathbf{k}\mathbf{m}^{-1}\mathbf{c}$$

or, equivalently, that $(\mathbf{m}^{-1}\mathbf{k})$ & $(\mathbf{m}^{-1}\mathbf{c})$ *commute*, i.e., $(\mathbf{m}^{-1}\mathbf{k})(\mathbf{m}^{-1}\mathbf{c}) = (\mathbf{m}^{-1}\mathbf{c})(\mathbf{m}^{-1}\mathbf{k})$.

Let Φ be the *modal matrix* and Ω^2 the *spectral matrix*. Specifically,

$$\Phi = [\Phi_1 \quad \Phi_2 \quad \cdots \quad \Phi_n \quad \cdots \quad \Phi_N] \quad \Omega^2 = \begin{bmatrix} \omega_1^2 & & & \\ & \omega_2^2 & & \\ & & \ddots & \\ & & & \omega_N^2 \end{bmatrix}$$

Invoking the *eigenvector expansion theorem*, we may express the structural response as $\mathbf{u}(t) = \Phi\mathbf{q}(t)$.

Then,

$$\begin{aligned} \mathbf{m}\ddot{\mathbf{u}} + \mathbf{c}\dot{\mathbf{u}} + \mathbf{k}\mathbf{u} = \mathbf{0} &\Rightarrow \mathbf{m}\Phi\ddot{\mathbf{q}}(t) + \mathbf{c}\Phi\dot{\mathbf{q}}(t) + \mathbf{k}\Phi\mathbf{q}(t) = \mathbf{0} \stackrel{\Phi^T}{\Leftrightarrow} \mathbf{m}\Phi\ddot{\mathbf{q}}(t) + \mathbf{c}\Phi\dot{\mathbf{q}}(t) + \mathbf{k}\Phi\mathbf{q}(t) = \mathbf{0} \\ &\quad \mathbf{m}\ddot{\mathbf{u}} + \mathbf{c}\dot{\mathbf{u}} + \mathbf{k}\mathbf{u} = \mathbf{0} \\ &\quad \downarrow \\ &\quad \mathbf{m}\Phi\ddot{\mathbf{q}}(t) + \mathbf{c}\Phi\dot{\mathbf{q}}(t) + \mathbf{k}\Phi\mathbf{q}(t) = \mathbf{0} \\ &\quad \downarrow \\ &\quad \Phi^T | \quad \mathbf{m}\Phi\ddot{\mathbf{q}}(t) + \mathbf{c}\Phi\dot{\mathbf{q}}(t) + \mathbf{k}\Phi\mathbf{q}(t) = \mathbf{0} \\ &\quad \downarrow \\ &\quad \underbrace{\Phi^T \mathbf{m} \Phi}_{\mathbf{M}} \ddot{\mathbf{q}}(t) + \underbrace{\Phi^T \mathbf{c} \Phi}_{\mathbf{C}} \dot{\mathbf{q}}(t) + \underbrace{\Phi^T \mathbf{k} \Phi}_{\mathbf{K}} \mathbf{q}(t) = \mathbf{0} \\ &\quad \downarrow \\ &\quad \mathbf{M}\ddot{\mathbf{q}}(t) + \mathbf{C}\dot{\mathbf{q}}(t) + \mathbf{K}\mathbf{q}(t) = \mathbf{0} \end{aligned}$$

where

$$\mathbf{M} = \Phi^T \mathbf{m} \Phi = \begin{bmatrix} M_1 & & & \\ & M_2 & & \\ & & \ddots & \\ & & & M_N \end{bmatrix} \quad \mathbf{C} = \Phi^T \mathbf{c} \Phi = \begin{bmatrix} C_1 & & & \\ & C_2 & & \\ & & \ddots & \\ & & & C_N \end{bmatrix} \quad \mathbf{K} = \Phi^T \mathbf{k} \Phi = \begin{bmatrix} K_1 & & & \\ & K_2 & & \\ & & \ddots & \\ & & & K_N \end{bmatrix}$$

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

$$M_n = \boldsymbol{\phi}_n^T \mathbf{m} \boldsymbol{\phi}_n \quad , \quad C_n = \boldsymbol{\phi}_n^T \mathbf{c} \boldsymbol{\phi}_n \quad , \quad K_n = \boldsymbol{\phi}_n^T \mathbf{k} \boldsymbol{\phi}_n$$

Therefore

$$M_n \ddot{q}_n + C_n \dot{q}_n + K_n q_n = 0$$

At this point we introduce the **damping ration of each mode**:

$$\xi_n = \frac{C_n}{2M_n\omega_n}$$

Therefore

$$\ddot{q}_n + 2\xi_n\omega_n\dot{q}_n + \omega_n^2q_n = 0 \quad (n = 1, 2, \dots, N)$$

Noticing that the above equation is mathematically identical to the equation that governs the free vibration response of a SDOF with damping, we may write

$$q_n(t) = e^{-\xi_n\omega_n t} \left[q_n(0) \cos(\omega_{Dn} t) + \frac{\dot{q}_n(0) + \xi_n\omega_n q_n(0)}{\omega_{Dn}} \sin(\omega_{Dn} t) \right]$$

where: $\omega_{Dn} = \omega_n \sqrt{1 - \xi_n^2}$, $q_n(0) = \frac{\boldsymbol{\phi}_n^T \mathbf{m} \mathbf{u}_0}{M_n}$, $\dot{q}_n(0) = \frac{\boldsymbol{\phi}_n^T \mathbf{m} \dot{\mathbf{u}}_0}{M_n}$

Recall that

$$\mathbf{u}(t) = \boldsymbol{\Phi} \mathbf{q}(t) = \sum_{n=1}^N q_n(t) \boldsymbol{\phi}_n$$

IMPORTANT NOTE: As for the undamped case, **for the structure to vibrate only in the n -th mode**, the initial displacement \mathbf{u}_0 and/or the initial velocity $\dot{\mathbf{u}}_0$ must both be proportional to $\boldsymbol{\phi}_n$, i.e., $\mathbf{u}_0 \sim \boldsymbol{\phi}_n$ & $\dot{\mathbf{u}}_0 \sim \boldsymbol{\phi}_n$.

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

APPENDIX

The *properties of the Generalized Characteristic-Value Problem* $\mathbf{k}\mathbf{u} = \lambda\mathbf{m}\mathbf{u}$ that we enumerated earlier may be demonstrated by following one of the following two strategies:

- (1) One may **work directly with** the *Generalized Characteristic-Value Problem* $\mathbf{k}\mathbf{u} = \lambda\mathbf{m}\mathbf{u}$, or
- (2) One may convert/transform the problem $\mathbf{k}\mathbf{u} = \lambda\mathbf{m}\mathbf{u}$ to an *equivalent Standard Eigenvalue Problem* of the form $\mathbf{A}\mathbf{v} = \lambda\mathbf{v}$, where the matrix \mathbf{A} is symmetric and positive definite.

Let us start by considering **strategy #1**:

We start by demonstrating that the characteristic values ($\lambda_r = \omega_r^2$) and modal shapes (ϕ_r) of a civil engineering structure are real.

Theorem

The **eigenvalues** λ of a Generalized Eigenvalue Problem $\mathbf{k}\mathbf{u} = \lambda\mathbf{m}\mathbf{u}$, with \mathbf{k} & \mathbf{m} **real & symmetric matrices**, and \mathbf{m} is **positive definite**, are real.

As a corollary, **the corresponding eigenvectors are also real**.

Proof

We consider the eigenvalue-eigenvector pair (λ_r, ϕ_r) and assume they are complex.

Because \mathbf{k} & \mathbf{m} are **real**, it follows that the complex conjugate pair $(\bar{\lambda}_r, \bar{\phi}_r)$ must also constitute an eigenvalue-eigenvector pair.

Therefore:

$$\left. \begin{array}{l} \mathbf{k}\phi_r = \lambda_r \mathbf{m}\phi_r \\ \mathbf{k}\bar{\phi}_r = \bar{\lambda}_r \mathbf{m}\bar{\phi}_r \end{array} \right\} \Rightarrow \left. \begin{array}{l} \bar{\phi}_r^T \mathbf{k}\phi_r = \lambda_r \bar{\phi}_r^T \mathbf{m}\phi_r \\ (\mathbf{k}\bar{\phi}_r)^T \phi_r = \bar{\lambda}_r (\mathbf{m}\bar{\phi}_r)^T \phi_r \end{array} \right\} \Rightarrow \left. \begin{array}{l} \bar{\phi}_r^T \mathbf{k}\phi_r = \lambda_r \bar{\phi}_r^T \mathbf{m}\phi_r \\ \bar{\phi}_r^T \mathbf{k}^T \phi_r = \bar{\lambda}_r \bar{\phi}_r^T \mathbf{m}^T \phi_r \end{array} \right\} \begin{array}{l} \text{Recall} \\ \Leftrightarrow \\ \mathbf{m} = \mathbf{m}^T \\ \mathbf{k} = \mathbf{k}^T \end{array}$$

$$\underbrace{\bar{\phi}_r^T \mathbf{k}\phi_r - \bar{\phi}_r^T \mathbf{k}^T \phi_r}_0 = (\lambda_r - \bar{\lambda}_r) \bar{\phi}_r^T \mathbf{m}\phi_r$$

Recalling that the **mass matrix** \mathbf{m} is a **real, symmetric**, and **positive definite** matrix, it is straight forward to demonstrate that $\bar{\phi}_r^T \mathbf{m}\phi_r$ is real and positive. Indeed,

$$\overline{(\bar{\phi}_r^T \mathbf{m}\phi_r)} = \phi_r^T \mathbf{m}\bar{\phi}_r = \phi_r^T \mathbf{m}^T \bar{\phi}_r = (\mathbf{m}\phi_r)^T \bar{\phi}_r = ((\mathbf{m}\phi_r)^T \bar{\phi}_r)^T = \bar{\phi}_r^T \mathbf{m}^T \phi_r = \bar{\phi}_r^T \mathbf{m}\phi_r$$

Therefore, $\bar{\phi}_r^T \mathbf{m}\phi_r$ is **real and positive**.

It follows that

$$(\lambda_r - \bar{\lambda}_r) = 0$$

Which can be satisfied if and only if λ_r is **real**.

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

The eigenvectors also can be taken to be real, by rejecting permissible complex multiplicative factors.

Then we proceed to prove that modal shapes ϕ_r & ϕ_s , corresponding to distinct eigenvalues, λ_r and λ_s , are mutually orthogonal relative to \mathbf{m} and \mathbf{k} .

Theorem

If ϕ_r & ϕ_s are eigenvectors, corresponding to two distinct eigenvalues λ_r and λ_s , respectively, of the eigenvalue problem $(\mathbf{k} - \lambda\mathbf{m})\phi = \mathbf{0}$, where \mathbf{k} & \mathbf{m} are real and symmetric matrices, and \mathbf{m} is positive definite, there follows:

$$\phi_r^T \mathbf{m} \phi_s = 0 \quad , \quad \phi_r^T \mathbf{k} \phi_s = 0 \quad , \quad \lambda_r \neq \lambda_s$$

We say that eigenvectors ϕ_r & ϕ_s are orthogonal (to each other) relative to \mathbf{k} & \mathbf{m} .

Proof

If λ_r & λ_s are distinct eigenvalues corresponding, respectively, to the eigenvectors ϕ_r & ϕ_s , there follows:

$$\mathbf{k}\phi_r = \lambda_r \mathbf{m}\phi_r \quad , \quad \mathbf{k}\phi_s = \lambda_s \mathbf{m}\phi_s$$

and hence, also:

$$(\mathbf{k}\phi_r)^T \phi_s = \lambda_r (\mathbf{m}\phi_r)^T \phi_s \quad , \quad \phi_r^T \mathbf{k}\phi_s = \lambda_s \phi_r^T \mathbf{m}\phi_s$$

or, making use of the symmetry in \mathbf{k} & \mathbf{m} ,

$$\phi_r^T \mathbf{k}\phi_s = \lambda_r \phi_r^T \mathbf{m}\phi_s \tag{1}$$

$$\phi_r^T \mathbf{k}\phi_s = \lambda_s \phi_r^T \mathbf{m}\phi_s \tag{2}$$

$$(2) - (1) \quad \Rightarrow \quad (\lambda_s - \lambda_r) \phi_r^T \mathbf{m}\phi_s = 0$$

Thus, since $\lambda_s \neq \lambda_r$ by assumption, we conclude that:

$$\left. \begin{array}{l} \phi_r^T \mathbf{m}\phi_s = 0 \\ \left(\frac{1}{\lambda_s}\right) \phi_r^T \mathbf{k}\phi_s = \phi_r^T \mathbf{m}\phi_s \end{array} \right\} \Rightarrow \phi_r^T \mathbf{k}\phi_s = 0$$

Finally, we demonstrate that the modal shapes ϕ_r are linearly independent, a fact that forms the basis of the *modal superposition method*.

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

Theorem

For mass matrix \mathbf{m} real, symmetric, and positive definite, any set of non-zero real vectors which are mutually orthogonal relative to \mathbf{m} is a set of linearly independent vectors.

Proof

The above Theorem may easily be demonstrated **by contradiction**. Indeed, let us assume that \mathbf{x}_i , ($i = 1, 2, \dots, \ell, \dots, S$) are S vectors that are mutually orthogonal relative to \mathbf{m} and let us assume they are linearly dependent, i.e., $\alpha_1 \mathbf{x}_1 + \alpha_2 \mathbf{x}_2 + \dots + \alpha_\ell \mathbf{x}_\ell + \dots + \alpha_S \mathbf{x}_S = \mathbf{0} \Rightarrow \mathbf{x}_\ell = -\sum_{i \neq \ell} (\alpha_i / \alpha_\ell) \mathbf{x}_i$. We pre-multiply by $\mathbf{x}_\ell^T \mathbf{m}$ and we exploit the orthogonality property: $\mathbf{x}_\ell^T \mathbf{m} \mathbf{x}_\ell = -\sum_{i \neq \ell} (\alpha_i / \alpha_\ell) \mathbf{x}_\ell^T \mathbf{m} \mathbf{x}_i = -\sum_{i \neq \ell} 0 = 0$. But this is a contradiction given that \mathbf{m} is positive definite and, consequently, $\mathbf{x}_\ell^T \mathbf{m} \mathbf{x}_\ell > 0$. Thus, by contradiction, the mutually orthogonal, relative to \mathbf{m} , vectors \mathbf{x}_i , ($i = 1, 2, \dots, \ell, \dots, S$) are **linearly independent**.

Now let us consider strategy #2:

A quick way to transform the *generalized eigenvalue problem* $\mathbf{k}\mathbf{u} = \lambda\mathbf{m}\mathbf{u}$ to the *standard eigenvalue problem* of the form $\mathbf{A}\mathbf{v} = \lambda\mathbf{v}$, is the following: $\mathbf{k}\mathbf{u} = \lambda\mathbf{m}\mathbf{u} \Leftrightarrow (\mathbf{m}^{-1}\mathbf{k})\mathbf{u} = \lambda\mathbf{u}$. However, the matrix $(\mathbf{m}^{-1}\mathbf{k})$ is not symmetric, and this obscures the problem. For this reason, we adopt a different approach.

We start with the following theorem:

Theorem

If matrix \mathbf{B} is real, symmetric and positive definite, then there exists a non-singular real matrix \mathbf{W} such that $\mathbf{B} = \mathbf{W}^T \mathbf{W}$

Proof

The matrix \mathbf{B} can be written as:

$$\mathbf{B} = \mathbf{R} \mathbf{D} \mathbf{R}^T$$

where:

- the **columns** of matrix \mathbf{R} consist of the orthonormal eigenvectors of \mathbf{B} , and
- $\mathbf{D} = \text{diag}(\mu_1, \mu_2, \dots, \mu_N)$, where the numbers $\mu_i > 0$ ($i = 1, 2, \dots, N$), are the corresponding eigenvalues of \mathbf{B} .

Indeed, let \mathbf{e}_r be the orthonormal eigenvectors of \mathbf{B} , i.e., $\mathbf{B}\mathbf{e}_r = \mu_r \mathbf{e}_r$. Then,

$$\mathbf{R} = \begin{pmatrix} \downarrow & \downarrow & \dots & \downarrow \\ \mathbf{e}_1 & \mathbf{e}_2 & \dots & \mathbf{e}_N \\ \downarrow & \downarrow & \dots & \downarrow \end{pmatrix}$$

Evidently

$$\mathbf{R}^T \mathbf{R} = \begin{pmatrix} \rightarrow & \mathbf{e}_1^T & \rightarrow \\ \rightarrow & \mathbf{e}_2^T & \rightarrow \\ \vdots & \vdots & \vdots \\ \rightarrow & \mathbf{e}_N^T & \rightarrow \end{pmatrix} \begin{pmatrix} \downarrow & \downarrow & \dots & \downarrow \\ \mathbf{e}_1 & \mathbf{e}_2 & \dots & \mathbf{e}_N \\ \downarrow & \downarrow & \dots & \downarrow \end{pmatrix} = \mathbf{I}$$

Therefore, $\mathbf{R}^T \mathbf{R} = \mathbf{I} \Leftrightarrow \mathbf{R}^T = \mathbf{R}^{-1} \Leftrightarrow (\mathbf{R}^T)^{-1} = \mathbf{R}$.

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

We may write

$$\mathbf{BR} = \mathbf{B} \begin{pmatrix} \downarrow & \downarrow & \cdots & \downarrow \\ \mathbf{e}_1 & \mathbf{e}_2 & \cdots & \mathbf{e}_N \\ \downarrow & \downarrow & \cdots & \downarrow \end{pmatrix} = \begin{pmatrix} \downarrow & \downarrow & \cdots & \downarrow \\ \mu_1 \mathbf{e}_1 & \mu_2 \mathbf{e}_2 & \cdots & \mu_N \mathbf{e}_N \\ \downarrow & \downarrow & \cdots & \downarrow \end{pmatrix} = \mathbf{RD}$$

It follows that: $\mathbf{BR} = \mathbf{RD} \Leftrightarrow \mathbf{BRR}^T = \mathbf{RDR}^T \Leftrightarrow \mathbf{BI} = \mathbf{RDR}^T \Leftrightarrow \mathbf{B} = \mathbf{RDR}^T$

Then:

$$\mathbf{B} = \mathbf{RDR}^T = \mathbf{R}\sqrt{\mathbf{D}}\sqrt{\mathbf{D}}\mathbf{R}^T = (\sqrt{\mathbf{D}}\mathbf{R}^T)^T (\sqrt{\mathbf{D}}\mathbf{R}^T) = \mathbf{W}^T\mathbf{W}$$

$$\text{where: } \boxed{\mathbf{W} = \sqrt{\mathbf{D}}\mathbf{R}^T}$$

Matrices, \mathbf{R}^T & \mathbf{R} , are **invertible** because they are **non-singular** (linear independence of eigenvectors).

Consequently, also \mathbf{W} is **non-singular**.

Notice that:

$$\begin{aligned} (\mathbf{W}^T)^{-1} &= (\mathbf{R}\sqrt{\mathbf{D}})^{-1} = (\sqrt{\mathbf{D}})^{-1}\mathbf{R}^{-1} \\ (\mathbf{W}^{-1})^T &= [(\mathbf{R}^T)^{-1}(\sqrt{\mathbf{D}})^{-1}]^T = (\sqrt{\mathbf{D}})^{-1}\mathbf{R}^T = (\sqrt{\mathbf{D}})^{-1}\mathbf{R}^{-1} \end{aligned}$$

Therefore:

$$\boxed{(\mathbf{W}^T)^{-1} = (\mathbf{W}^{-1})^T}$$

Using the above Theorem, the mass matrix \mathbf{m} , being a real, symmetric and positive definite matrix, may be resolved as follows:

$$\mathbf{m} = \mathbf{Q}^T\mathbf{Q}$$

where \mathbf{Q} is a **real, nonsingular** matrix.

Therefore:

$$\mathbf{k}\boldsymbol{\phi} = \lambda\mathbf{m}\boldsymbol{\phi} \Rightarrow \mathbf{k}\boldsymbol{\phi} = \lambda(\mathbf{Q}^T\mathbf{Q})\boldsymbol{\phi} \quad (1)$$

Next, we consider the **linear transformation**:

$$\boxed{\mathbf{Q}\boldsymbol{\phi} = \mathbf{v}} \quad (2)$$

from which we obtain the **inverse transformation**:

$$\boldsymbol{\phi} = \mathbf{Q}^{-1}\mathbf{v} \quad (3)$$

The inverse \mathbf{Q}^{-1} is guaranteed to exist because \mathbf{Q} is **non-singular**.

Introducing (2) & (3) into (1) and pre-multiplying on the left by $(\mathbf{Q}^T)^{-1}$, we obtain the eigenvalue problem:

$$\boxed{\mathbf{A}\mathbf{v} = \lambda\mathbf{v}}$$

where, considering the relation $(\mathbf{Q}^T)^{-1} = (\mathbf{Q}^{-1})^T$, we conclude that:

$$\mathbf{A} = (\mathbf{Q}^T)^{-1}\mathbf{k}\mathbf{Q}^{-1} = (\mathbf{Q}^{-1})^T\mathbf{k}\mathbf{Q}^{-1} = \mathbf{A}^T$$

i.e., matrix $\mathbf{A} = (\mathbf{Q}^T)^{-1}\mathbf{k}\mathbf{Q}^{-1} = (\mathbf{Q}^{-1})^T\mathbf{k}\mathbf{Q}^{-1}$ is a **real, symmetric** matrix.

It is evident that the original $\mathbf{k}\boldsymbol{\phi} = \lambda\mathbf{m}\boldsymbol{\phi}$ has the **same eigenvalues** as the problem $\mathbf{A}\mathbf{v} = \lambda\mathbf{v}$ (with $\mathbf{A} = (\mathbf{Q}^T)^{-1}\mathbf{k}\mathbf{Q}^{-1} = (\mathbf{Q}^{-1})^T\mathbf{k}\mathbf{Q}^{-1}$) and the **eigenvectors** are related as follows: $\mathbf{Q}\boldsymbol{\phi} = \mathbf{v} \Leftrightarrow \boldsymbol{\phi} = \mathbf{Q}^{-1}\mathbf{v}$.

PART (11): MDOF SYSTEMS – FREE VIBRATION RESPONSE – EIGENVALUE PROBLEM

Furthermore, the orthogonality properties of the eigenvectors of one problem imply the orthogonality properties of the eigenvectors of the other problem. Specifically,

$$\text{'m-orthogonality'} \quad \boldsymbol{\phi}_i^T \mathbf{m} \boldsymbol{\phi}_j = \boldsymbol{\phi}_i^T \mathbf{Q}^T \mathbf{Q} \boldsymbol{\phi}_j = \mathbf{v}_i^T \mathbf{v}_j = 0 \quad (i \neq j), \text{ which leads to:}$$

$$\text{'k-orthogonality'} \quad \boldsymbol{\phi}_i^T \mathbf{k} \boldsymbol{\phi}_j = \lambda_j \boldsymbol{\phi}_i^T \mathbf{m} \boldsymbol{\phi}_j = 0 \quad (i \neq j)$$

The real eigenvalues λ_r , ($r = 1, 2, \dots, N$) of the problem $\mathbf{k}\boldsymbol{\phi} = \lambda\mathbf{m}\boldsymbol{\phi}$ are all positive because $\boldsymbol{\phi}_r^T \mathbf{m} \boldsymbol{\phi}_r > 0$ and $\boldsymbol{\phi}_r^T \mathbf{k} \boldsymbol{\phi}_r > 0$. Specifically,

$$\mathbf{k}\boldsymbol{\phi}_r = \lambda_r \mathbf{m}\boldsymbol{\phi}_r \Leftrightarrow \boldsymbol{\phi}_r^T \mathbf{k} \boldsymbol{\phi}_r = \lambda_r \boldsymbol{\phi}_r^T \mathbf{m} \boldsymbol{\phi}_r \Leftrightarrow \lambda_r = (\boldsymbol{\phi}_r^T \mathbf{k} \boldsymbol{\phi}_r) / (\boldsymbol{\phi}_r^T \mathbf{m} \boldsymbol{\phi}_r) > 0$$

For completeness we remind the reader that the eigenvalues λ_r are also the eigenvalues of the problem $\mathbf{A}\mathbf{v} = \lambda\mathbf{v}$ [with $\mathbf{A} = (\mathbf{Q}^{-1})^T \mathbf{k} \mathbf{Q}^{-1}$], which implies that matrix \mathbf{A} , besides being **real** and **symmetric**, is also **positive definite**.

NOTE: The relationship $(\mathbf{Q}^T)^{-1} = (\mathbf{Q}^{-1})^T$ is obtained as follows:

$$\mathbf{I} = (\mathbf{I})^T \Leftrightarrow \mathbf{I} = (\mathbf{Q}^{-1} \mathbf{Q})^T \Leftrightarrow \mathbf{I} = \mathbf{Q}^T (\mathbf{Q}^{-1})^T \Leftrightarrow (\mathbf{Q}^T)^{-1} = (\mathbf{Q}^{-1})^T$$

NOTE:

The **orthogonality property** of eigenvectors, demonstrated above, was based on the assumption that the corresponding **eigenvalues** are **distinct**.

The question arises as to **what happens when there are repeated eigenvalues**, *i.e.*, when two or more eigenvalues have the same value, and we note that **when an eigenvalue λ_i is repeated m_i times**, where m_i is integer, λ_i is **said to have multiplicity m_i** .

The answer to the above question lies in the following **Theorem**:

Theorem

If an eigenvalue λ_i of a real-symmetric matrix \mathbf{A} has multiplicity m_i , then \mathbf{A} has exactly m_i linearly independent eigenvectors corresponding to λ_i .

[For a proof see section 1.18 of HILDEBRAND, F.B. (1965), *Methods of Applied Mathematics*, 2nd Edition.]

These eigenvectors are **not unique**, as **any linear combination of the eigenvectors belonging to a repeated eigenvalue is also an eigenvector**.

The linearly independent eigenvectors corresponding to λ_i are **not necessarily orthogonal**. **Any set of linearly independent eigenvectors, though, can be rendered orthogonal by a procedure known as the Gram-Schmidt orthogonalization process**.

Of course, the eigenvectors belonging to the repeated eigenvalue are orthogonal to the eigenvectors belonging to the remaining eigenvalues. Hence, **all the eigenvectors of a real symmetric matrix \mathbf{A} are orthogonal regardless of whether there are repeated eigenvalues or not**.