

NUMERICAL EVALUATION OF DYNAMIC RESPONSE

Αριθμητικός Υπολογισμός Δυναμικής Απόκρισης

TIME-STEPPING (or DIFFERENCE) METHODS:

Μέθοδοι Χρονικών Βημάτων

Equation of Motion of an inelastic system:

$$m\ddot{u} + c\dot{u} + f_S(u, \dot{u}) = p(t)$$

Initial Conditions (I. C. 's): $u_0 = u(0)$ & $\dot{u}_0 = \dot{u}(0)$

Important requirements for a numerical procedure:

- (1) **Convergence:** **Σύγκλιση**
As the time step decreases, **the numerical solution should approach the exact solution.**
- (2) **Stability:** **Ευστάθεια**
The numerical solution **should be stable in the presence of numerical round-off errors.**
- (3) **Accuracy:** **Ακρίβεια**
The numerical procedure **should provide results that are close enough to the exact solution.**

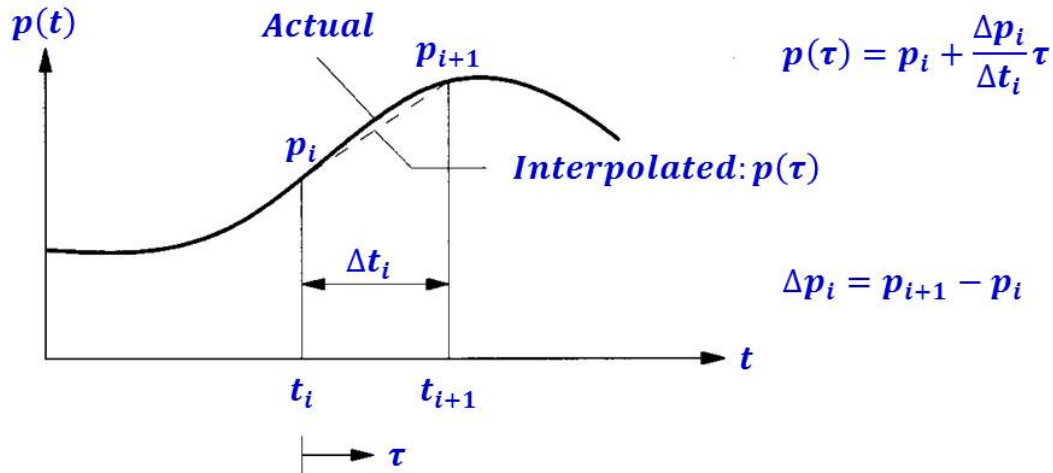
Types of time-stepping procedures:

- (1) Methods based on **interpolation of excitation function**. **Μέθοδοι βασισμένοι στην παρεμβολή της διέγερσης**
- (2) Methods based on **finite difference expressions of velocity & acceleration**. **Μέθοδος κεντρικής διαφοράς**
- (3) Methods based on **assumed variation of acceleration**. **Μέθοδοι βασισμένοι στην υπόθεση μεταβολής της επιταχύνσεως**

METHODS BASED ON INTERPOLATION OF EXCITATION (LINEAR SYSTEMS)

Interpolation of the excitation over each time interval

For sufficiently short time intervals, **linear interpolation**:



Response $u(\tau)$ for $0 \leq \tau \leq \Delta t_i$:

(1) Free vibration due to **I.C.'s** u_i & \dot{u}_i at $\tau = 0$

+

(2) Response to **step force** p_i with **zero initial conditions**

+

(3) Response to **ramp force** $\left(\frac{\Delta p_i}{\Delta t_i} \tau\right)$ with **zero initial conditions**

Each of the above responses is available in closed form for linear systems. Therefore:

$$\begin{pmatrix} u \\ \dot{u} \end{pmatrix}_{i+1} = \begin{pmatrix} A & B \\ A' & B' \end{pmatrix} \begin{pmatrix} u \\ \dot{u} \end{pmatrix}_i + \begin{pmatrix} C & D \\ C' & D' \end{pmatrix} \begin{pmatrix} p_i \\ p_{i+1} \end{pmatrix}$$

Expressions for $A, B, A', B', C, D, C', D'$ are given in TABLE 5.2.1 of CHOPRA (1995).

The above described procedure is used to compute **elastic response spectra**.

METHODS BASED ON DIFFERENCE EXPRESSIONS**Central Difference Method** Μέθοδος Κεντρικής Διαφοράς

Approximate \dot{u} & \ddot{u} by difference equations:

$$\dot{u}_i = \frac{u_{i+1} - u_{i-1}}{2(\Delta t)} \quad ; \quad \ddot{u}_i = \frac{u_{i+1} - 2u_i + u_{i-1}}{(\Delta t)^2}$$

One way of obtaining these formulae is by **Taylor expansion**:

$$u(t + \Delta t) = u(t) + (\Delta t)\dot{u}(t) + \frac{(\Delta t)^2}{2!}\ddot{u}(t) + \frac{(\Delta t)^3}{3!}\ddot{\ddot{u}}(\xi_1) \quad (1)$$

$$u(t - \Delta t) = u(t) - (\Delta t)\dot{u}(t) + \frac{(\Delta t)^2}{2!}\ddot{u}(t) - \frac{(\Delta t)^3}{3!}\ddot{\ddot{u}}(\xi_2) \quad (2)$$

Subtracting (2) from (1), we obtain:

$$\begin{aligned} u(t + \Delta t) - u(t - \Delta t) &= 2(\Delta t)\dot{u}(t) + \frac{(\Delta t)^3}{3!} \underbrace{[\ddot{\ddot{u}}(\xi_1) + \ddot{\ddot{u}}(\xi_2)]}_{=2\ddot{\ddot{u}}(\xi)} \\ \Rightarrow \dot{u}(t) &= \frac{u(t + \Delta t) - u(t - \Delta t)}{2(\Delta t)} - \frac{(\Delta t)^2}{6}\ddot{\ddot{u}}(\xi) \end{aligned}$$

Add (2) to (1) after expanding up to 4th power:

$$\begin{aligned} u(t + \Delta t) + u(t - \Delta t) &= 2u(t) + (\Delta t)^2\ddot{u}(t) + \frac{(\Delta t)^4}{4!} \underbrace{\left[\frac{d^4u(\xi_1)}{dt^4} + \frac{d^4u(\xi_2)}{dt^4} \right]}_{=2\frac{d^4u(\xi)}{dt^4}} \\ \Rightarrow \ddot{u}(t) &= \frac{u(t + \Delta t) - 2u(t) + u(t - \Delta t)}{(\Delta t)^2} - \frac{(\Delta t)^2}{12} \left(\frac{d^4u(\xi)}{dt^4} \right) \end{aligned}$$

Equation of Motion at step i :

$$m\ddot{u}_i + c\dot{u}_i + (f_S)_i = p_i$$

Substituting the approximate expression for \dot{u}_i & \ddot{u}_i :

$$m \frac{u_{i+1} - 2u_i + u_{i-1}}{(\Delta t)^2} + c \frac{u_{i+1} - u_{i-1}}{2(\Delta t)} + (f_S)_i = p_i$$

Transferring the unknown quantities to the right side, we obtain:

$$\text{where: } \begin{cases} \hat{k}u_{i+1} = \hat{p}_i \Leftrightarrow u_{i+1} = \frac{\hat{p}}{\hat{k}} \\ \hat{k} = \frac{m}{(\Delta t)^2} + \frac{c}{2(\Delta t)} \\ \hat{p} = p_i - \left[\frac{m}{(\Delta t)^2} - \frac{c}{2(\Delta t)} \right] u_{i-1} - (f_S)_i + \frac{2m}{(\Delta t)^2} u_i \end{cases}$$

(Explicit Method)

(Ρητή Μέθοδος)

To initialize the process:

$$\dot{u}_0 = \frac{u_1 - u_{-1}}{2(\Delta t)} \quad ; \quad \ddot{u}_0 = \frac{u_1 - 2u_0 + u_{-1}}{(\Delta t)^2}$$

After eliminating u_1 , we solve for u_{-1} : $u_{-1} = u_0 - (\Delta t)\dot{u}_0 + \frac{(\Delta t)^2}{2}\ddot{u}_0$

u_0 & \dot{u}_0 are given initial conditions

Equation of Motion: $m\ddot{u}_0 + c\dot{u}_0 + (f_S)_0 = p_0 \Rightarrow \ddot{u}_0 = \frac{1}{m}[p_0 - c\dot{u}_0 - (f_S)_0]$

Stability requirement:

$$\frac{(\Delta t)}{T} < \frac{1}{\pi}$$

Typically, $\frac{(\Delta t)}{T} \leq 0.1$, (T = natural period of the SDOF system) to define the response adequately.

NEWMARK's β METHOD ΜΕΘΟΔΟΣ NEWMARK

'*Newmark's Method*' comprises a family of time-stepping methods based on the following equations:

$$\begin{aligned} \dot{u}_{i+1} &= \dot{u}_i + [(1 - \gamma)\Delta t]\ddot{u}_i + (\gamma\Delta t)\ddot{u}_{i+1} \\ u_{i+1} &= u_i + (\Delta t)\dot{u}_i + \left[\frac{1-\beta}{2}\right](\Delta t)^2\ddot{u}_i + [\beta(\Delta t)^2]\ddot{u}_{i+1} \end{aligned}$$

The parameters β & γ :

- define **the variation of acceleration** over (Δt)
- determine the **stability & accuracy** characteristics

Method is **implicit** \Rightarrow iteration

For linear systems, the method may be modified to become **explicit**.

Typical selection of parameters:

$$\gamma = \frac{1}{2} \quad \frac{1}{6} \leq \beta \leq \frac{1}{4}$$

for satisfactory performance, including accuracy.

Special Cases:

- (1) $\gamma = 0 \quad \beta = 0$: **Constant acceleration method**
- (2) $\gamma = \frac{1}{2} \quad \beta = \frac{1}{4}$: **Average acceleration method**
- (3) $\gamma = \frac{1}{2} \quad \beta = \frac{1}{6}$: **Linear acceleration method**

Newmark's method is **stable if:**

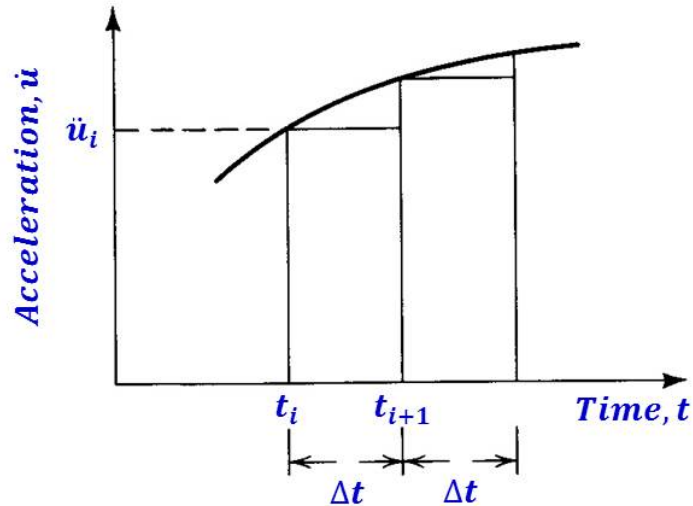
$$\left(\frac{\Delta t}{T}\right) \leq \frac{1}{\pi\sqrt{2}} \frac{1}{\sqrt{\gamma - 2\beta}}$$

For: $\gamma = \frac{1}{2} \quad \beta = \frac{1}{4} \Rightarrow \frac{\Delta t}{T} < \infty$

For: $\gamma = \frac{1}{2} \quad \beta = \frac{1}{6} \Rightarrow \frac{\Delta t}{T} \leq 0.551$

Constant-Acceleration Method: ($\gamma = 0$ $\beta = 0$)

$$\begin{aligned} \ddot{u}(\tau) &= \ddot{u} \\ \dot{u}(\tau) &= \dot{u}_i + \tau \ddot{u}_i & \Rightarrow & \boxed{\dot{u}_{i+1} = \dot{u}_i + (\Delta t) \ddot{u}_i} \\ u(\tau) &= u_i + \tau \dot{u}_i + \frac{\tau^2}{2} \ddot{u}_i & \Rightarrow & \boxed{u_{i+1} = u_i + (\Delta t) \dot{u}_i + \frac{(\Delta t)^2}{2} \ddot{u}_i} \end{aligned}$$



The above two equations provide two of the three equations for time integration, *i.e.* to obtain u_{i+1} , \dot{u}_{i+1} , \ddot{u}_{i+1} from u_i , \dot{u}_i , \ddot{u}_i .

The third equation is the **Equation of Motion**:

$$m\ddot{u}_{i+1} + c\dot{u}_{i+1} + ku_{i+1} = p_{i+1}$$

Then:

$$\ddot{u}_{i+1} = \frac{1}{m} \left\{ p_{i+1} - ku_i - [c + k(\Delta t)]\dot{u}_i - \left[c(\Delta t) + k \frac{(\Delta t)^2}{2} \right] \ddot{u}_i \right\}$$

To begin the time integration, we need to know the values of u_0 , \dot{u}_0 & \ddot{u}_0 , *i.e.* the values of displacement, velocity and acceleration at $t = 0$.

Two of them must be specified; the third is obtained by using the Equation of Motion at $t = 0$.

Average Acceleration Method: $(\gamma = \frac{1}{2} \quad \beta = \frac{1}{4})$

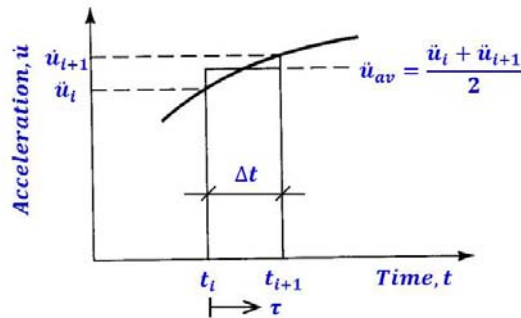
$$\ddot{u}(\tau) = \frac{1}{2}(\ddot{u}_{i+1} + \ddot{u}_i)$$

$$\dot{u}(\tau) = \dot{u}_i + \tau \frac{(\ddot{u}_{i+1} + \ddot{u}_i)}{2}$$

$$\Rightarrow \boxed{\dot{u}_{i+1} = \dot{u}_i + (\Delta t) \frac{(\ddot{u}_{i+1} + \ddot{u}_i)}{2}}$$

$$u(\tau) = u_i + \tau \dot{u}_i + \left(\frac{\tau^2}{2}\right) \frac{(\ddot{u}_{i+1} + \ddot{u}_i)}{2}$$

$$\Rightarrow \boxed{u_{i+1} = u_i + (\Delta t)\dot{u}_i + \frac{(\Delta t)^2}{4}(\ddot{u}_{i+1} + \ddot{u}_i)}$$



Linear Acceleration: $(\gamma = \frac{1}{2} \quad \beta = \frac{1}{6})$

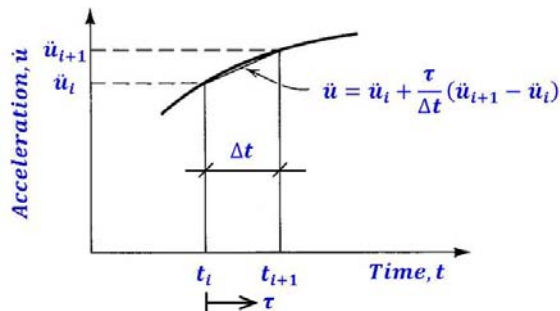
$$\ddot{u}(\tau) = \ddot{u}_i + \frac{\tau}{(\Delta t)}(\ddot{u}_{i+1} - \ddot{u}_i)$$

$$\dot{u}(\tau) = \dot{u}_i + \tau \ddot{u}_i + \frac{\tau^2}{2(\Delta t)}(\ddot{u}_{i+1} - \ddot{u}_i)$$

$$\Rightarrow \boxed{\dot{u}_{i+1} = \dot{u}_i + (\Delta t) \frac{(\ddot{u}_{i+1} + \ddot{u}_i)}{2}}$$

$$u(\tau) = u_i + \tau \dot{u}_i + \left(\frac{\tau^2}{2}\right) \ddot{u}_i + \left(\frac{\tau^3}{6(\Delta t)}\right) (\ddot{u}_{i+1} - \ddot{u}_i)$$

$$\Rightarrow \boxed{u_{i+1} = u_i + (\Delta t)\dot{u}_i + (\Delta t)^2 \left(\frac{1}{6}\ddot{u}_{i+1} + \frac{1}{3}\ddot{u}_i\right)}$$



Non-iterative Formulation of Newmark's β Method (Linear Systems):Μη επαναληπτική (ή μη θαμιστική) διατύπωση

Let:

$$\begin{aligned}\Delta u_i &\stackrel{\text{def}}{=} u_{i+1} - u_i \\ \Delta \dot{u}_i &\stackrel{\text{def}}{=} \dot{u}_{i+1} - \dot{u}_i \\ \Delta \ddot{u}_i &\stackrel{\text{def}}{=} \ddot{u}_{i+1} - \ddot{u}_i \quad \Delta p_i = p_{i+1} - p_i\end{aligned}$$

Newmark's time-stepping equations: Εξισώσεις χρονικών βημάτων

$$\Delta \dot{u}_i = (\Delta t)\ddot{u}_i + \gamma(\Delta t)\Delta \ddot{u}_i \quad (1a)$$

$$\Delta u_i = (\Delta t)\dot{u}_i + \frac{(\Delta t)^2}{2}\ddot{u}_i + \beta(\Delta t)^2\Delta \ddot{u}_i \quad (1b)$$

$$(1b) \Rightarrow \Delta \ddot{u}_i = \frac{1}{\beta(\Delta t)^2}\Delta u_i - \frac{1}{\beta(\Delta t)}\dot{u}_i - \frac{1}{2\beta}\ddot{u}_i \quad (2)$$

$$(2) + (1a) \Rightarrow \Delta \dot{u}_i = \frac{\gamma}{\beta(\Delta t)}\Delta u_i - \frac{\gamma}{\beta}\dot{u}_i + (\Delta t)\left(1 - \frac{\gamma}{2\beta}\right)\ddot{u}_i \quad (3)$$

Incremental Equation of Motion: Αυξητική εξίσωση κίνησης

$$m\Delta \ddot{u}_i + c\Delta \dot{u}_i + k\Delta u_i = \Delta p_i \quad (4)$$

$$(2) + (3) + (4) \Rightarrow \hat{k}\Delta u_i = \Delta \hat{p}_i$$

where: $\hat{k} = k + \frac{\gamma}{\beta(\Delta t)}c + \frac{1}{\beta(\Delta t)^2}m$

and $\Delta \hat{p}_i = \Delta p_i + \left(\frac{1}{\beta(\Delta t)}m + \frac{\gamma}{\beta}c\right)\dot{u}_i + \left[\frac{1}{2\beta}m + (\Delta t)\left(\frac{\gamma}{2\beta} - 1\right)c\right]\ddot{u}_i$

Therefore: $\left. \begin{array}{l} \Delta u_i = \frac{\Delta \hat{p}_i}{\hat{k}} \\ \text{Equation (3)} \end{array} \right\} \Rightarrow \Delta \dot{u}_i \Rightarrow \begin{cases} u_{i+1} \\ \dot{u}_{i+1} \end{cases}$

$$\ddot{u}_{i+1} = \frac{p_{i+1} - c\dot{u}_{i+1} - ku_{i+1}}{m} \Rightarrow \ddot{u}_{i+1}$$

Implicit Method (Πεπλεγμένη Μέθοδος)

ERRORS INVOLVED IN NUMERICAL INTEGRATION

Types of errors:

- (1) **Round-off errors** due to precision of the floating point arithmetic of computers.
Αριθμητικό σφάλμα στρογγυλοποίησης
- (2) **Truncation errors** involved in representing u_{i+1} or \dot{u}_{i+1} by a finite number of terms in the Taylor series expansion. This error is represented analytically by the term $C(\Delta t)^p \left(\frac{d^p u(\xi)}{dt^p} \right)$. **Αριθμητικό σφάλμα περικοπής**

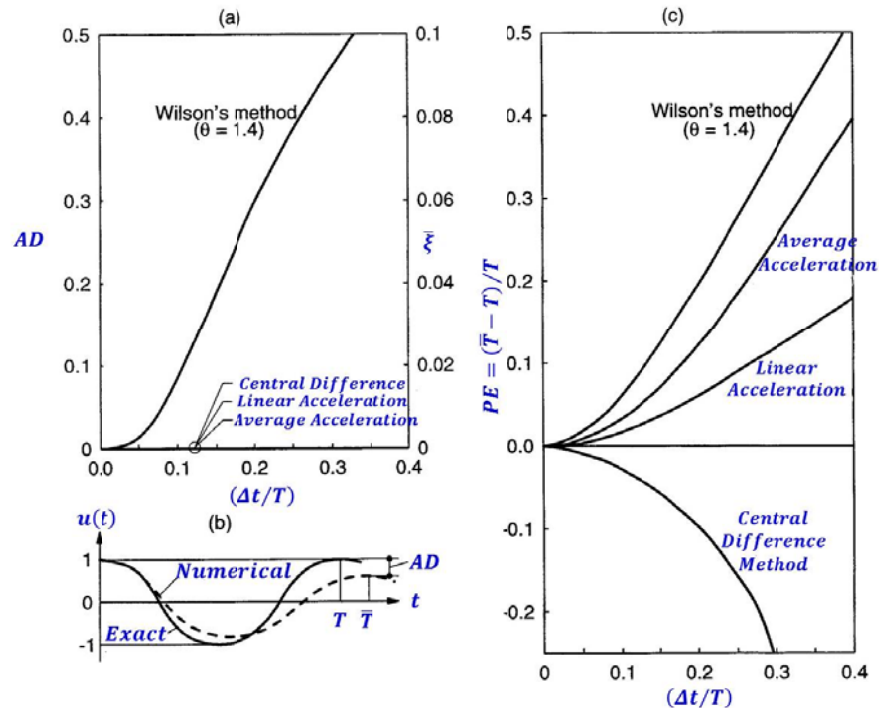
A very important aspect in the error analysis of numerical methods is the **growth** or **accumulation of errors** as computation progresses (**error propagation**).

This consideration is related to **the stability** of the numerical scheme.

The performance of a numerical scheme is evaluated by examining two important characteristics:

- (1) **Amplitude decay (AD)** as a function of $\left(\frac{\Delta t}{T}\right)$ **Εξασθένηση Εύρους Ταλάντωσης**
- (2) **Period Elongation (PE)** as a function of $\left(\frac{\Delta t}{T}\right)$ **Επιμήκυνση Περιόδου**

Amplitude decay and period elongation



The **Central Difference, Linear Acceleration & Average Acceleration Methods** introduce **no artificial damping**, i.e. $AD = 0$.

ANALYSIS OF NONLINEAR RESPONSE**ΑΝΑΛΥΣΗ ΜΗ ΓΡΑΜΜΙΚΗΣ ΑΠΟΚΡΙΣΗΣ**

Equation of Motion:
$$m\ddot{u} + c\dot{u} + f_S(u, \dot{u}) = p(t)$$

$$t = t_i: \quad m\ddot{u}_i + c\dot{u}_i + (f_S)_i = p_i$$

$$t = t_{i+1}: \quad m\ddot{u}_{i+1} + c\dot{u}_{i+1} + (f_S)_{i+1} = p_{i+1}$$

$$m(\ddot{u}_{i+1} - \ddot{u}_i) + c(\dot{u}_{i+1} - \dot{u}_i) + [(f_S)_{i+1} - (f_S)_i] = (p_{i+1} - p_i)$$

or
$$\boxed{m\Delta\ddot{u}_i + c\Delta\dot{u}_i + (\Delta f_S)_i = \Delta p_i}$$

The incremental resisting force:

$$(\Delta f_S)_i = (k_i)_{sec} \cdot \Delta u_i$$

where: $(k_i)_{sec} =$ **secant stiffness** (not known) **τέμνουσα δυσκαμψία**

or approximately:

$$(\Delta f_S)_i \cong (k_i)_T \cdot \Delta u_i$$

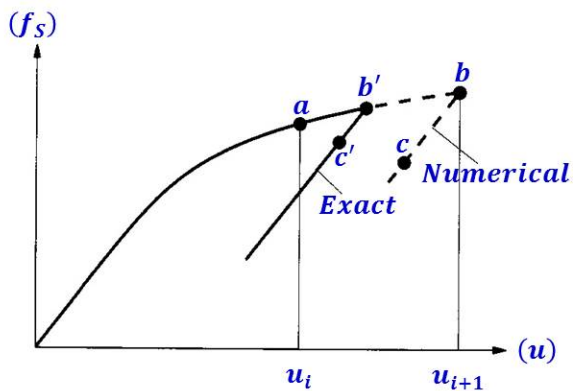
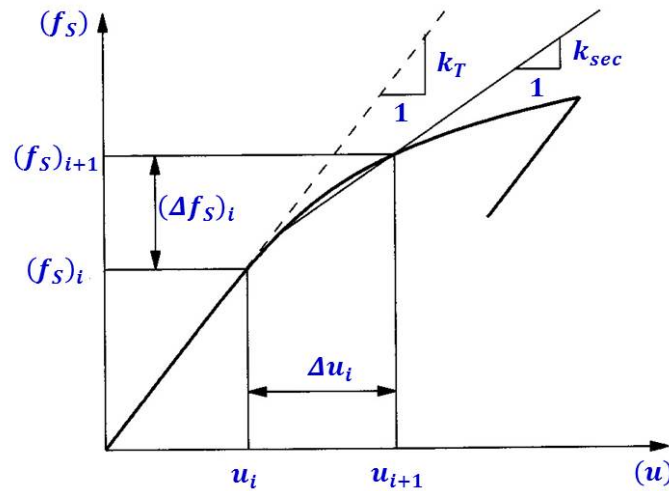
where: $(k_i)_T =$ **tangent stiffness** **εφαπτομενική δυσκαμψία**Application of **Newmark's Method**, which is the most popular method because of its accuracy:

where:
$$\begin{cases} \hat{k}_i \cdot \Delta u_i = \Delta \hat{p}_i \\ \hat{k}_i = (k_i)_T + \frac{\gamma}{\beta(\Delta t)} c + \frac{1}{\beta(\Delta t)^2} m \\ \Delta \hat{p}_i = \Delta p_i + \left(\frac{1}{\beta(\Delta t)} m + \frac{\gamma}{\beta} c \right) \dot{u}_i + \left[\frac{1}{2\beta} m + (\Delta t) \left(\frac{\gamma}{2\beta} - 1 \right) c \right] \ddot{u}_i \end{cases}$$

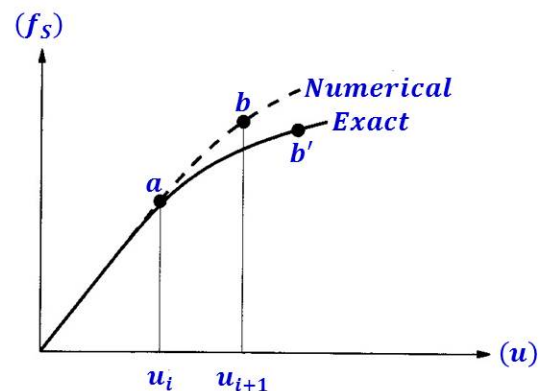
i.e., same formulae as in the linear case except that k has been replaced by $(k_i)_T$.

Significant error arises for **two** reasons:

- (1) The **tangent stiffness** is used instead of the **secant stiffness**;
- (2) Use of a constant time step delays detection of the transition in the force deformation relation.



Error (2)

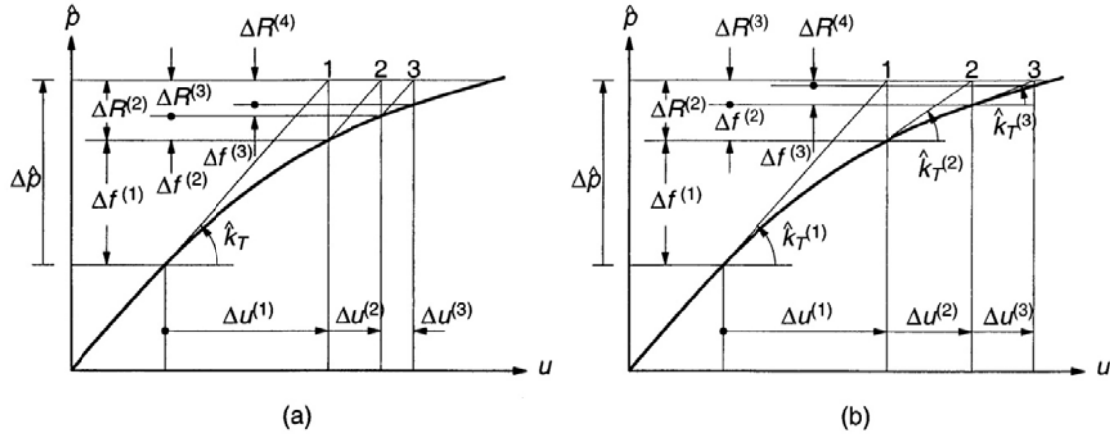


Error (1)

Error (2) is eliminated by using an **iterative process** in which integration is resumed from t_i with a smaller step whose size is **progressively adjusted** so that **at the end of such an adjusted time step, the velocity is close to zero**.

Error (1) is eliminated by an **iteration process** (**θαμιστική μέθοδος**) that is known as **Newton-Raphson Method**.

Newton-Raphson iteration schemes



Initialize data:

$$u_{i+1}^{(0)} = u_i \quad f_s^{(0)} = (f_s)_i \quad \Delta R^{(1)} = \Delta \hat{p}_i$$

Calculation for each iteration: $j = 1, 2, 3, \dots$

$$\begin{aligned} \hat{k}_i \Delta u^{(j)} &= \Delta R^{(j)} \Rightarrow \Delta u^{(j)} \\ u_{i+1}^{(j)} &= u_{i+1}^{(j-1)} + \Delta u^{(j)} \\ \Delta f^{(j)} &= f_s^{(j)} - f_s^{(j-1)} + \left[\frac{\gamma}{\beta(\Delta t)} c + \frac{1}{\beta(\Delta t)^2} m \right] \Delta u^{(j)} \\ \Delta R^{(j+1)} &= \Delta R^{(j)} - \Delta f^{(j)} \end{aligned}$$

LINEAR MULTISTEP (LMS) METHODS*LMS Methods for 1st-order Equations:*

Consider a system of 1st-order Ordinary Differential Equations (ODE's):

$$\boxed{\dot{\mathbf{y}} = \mathbf{f}(\mathbf{y}, t)} \quad (1)$$

A ***k*-step linear multistep method** for equation (1) is defined by the following expression:

$$\sum_{i=0}^k \{\alpha_i \mathbf{y}_{n+1-i} + (\Delta t) \beta_i \mathbf{f}(\mathbf{y}_{n+1-i}, t_{n+1-i})\} = \mathbf{0} \quad (2)$$

The α_i & β_i are parameters that define the method.

Note that the word '**linear**', in '**linear multistep method**', has nothing to do with the linearity of equation (1). Indeed, equation (2) is perfectly well defined for a nonlinear function $\mathbf{f}(\mathbf{y}, t)$.

Excellent references for **LMS methods** of this type are:

GEAR, C.W. (1971), *Numerical Initial Value Problems in Ordinary Differential Equations*, Prentice-Hall, Englewood Cliffs, NJ.

ISERLES, A. (1996), *A First Course in the Numerical Analysis of Differential Equations*, Cambridge University Press.

EXAMPLE:

The equation of motion of the linear, viscously damped, SDOF system can be cast in 1st-order form:

$$m\ddot{u} + c\dot{u} + ku = p(t) \quad \Rightarrow \quad \ddot{u} + 2\xi\omega\dot{u} + \omega^2u = \left(\frac{1}{m}\right)p(t)$$

Let:

$$\begin{aligned} y_1 &= u \\ y_2 &= \dot{u} \end{aligned}$$

Then: *Equation of Motion* $\Rightarrow \dot{y}_2 = \left(\frac{1}{m}\right)p(t) - 2\xi\omega y_2 - \omega^2 y_1$

Therefore:

$$\begin{pmatrix} \dot{y}_1 \\ \dot{y}_2 \end{pmatrix} = \begin{pmatrix} 0 & 1 \\ -\omega^2 & -2\xi\omega \end{pmatrix} \begin{pmatrix} y_1 \\ y_2 \end{pmatrix} + \begin{pmatrix} 0 \\ \left(\frac{1}{m}\right)p(t) \end{pmatrix} = \begin{pmatrix} f_1(\mathbf{y}, t) \\ f_2(\mathbf{y}, t) \end{pmatrix}$$

i.e.

$$\dot{\mathbf{y}} = \mathbf{f}(\mathbf{y}, t)$$

Multistep Methods:

Runge-Kutta (one step method)

Adams-Bashford

Adams-Multon

Predictor-Corrector Methods